## MECHANICAL PROPERTIES OF MULTI-YEAR SEA ICE, PHASE II

Progress Report 2

April 1983

bу

G.F.N. Cox, J.A. Richter, W.F. Weeks and M. Mellor
U.S. Army Cold Regions Research and Engineering Laboratory
Hanover, NH 03755

Prepared for
Shell Development Company
Minerals Management Service

### INTRODUCTION

This progress report presents the results of the constant strain-rate triaxial tests which were performed during the months of January and February, 1983.

## TRIAXIAL TESTS

Conventional triaxial tests were performed on the closed-loop testing machine using sample preparation and testing techniques similar to those employed in Phase I. As a result of our experience in Phase I, the triaxial cell was modified to increase its load bearing capacity to 350 kN (80,000 lbs) and confining pressure capacity to 24 MPa (3500 lbf/in.<sup>2</sup>). Heavier latex membranes were also placed around the sample to prevent penetration of hydraulic fluid into the sample. A 22 kN (100,000 lb) load cell was provided by Shell to measure axial forces in excess of 11 kN (50,000 lb). The upper cylinder was also modified such that tests could be performed at confining pressure/axial stress ratios of 0.25 and 0.50.

A total of 55 triaxial tests were performed on multi-year pressure ridge samples at different test temperatures, strain-rates, and confining pressures. The number of tests at each test condition is summarized in Table 1. In Phase I, triaxial tests were performed on multi-year floe samples at axial stress/confining pressure ratios of 0.46 and 0.64 at the same temperatures and strain-rates.

#### Triaxial Strength

A detailed tabulation of the results from the triaxial tests is given in Appendix A. The average confined compresive strength of the ice,  $\sigma_1$ , for each test condition is plotted against the confining pressure ( $\sigma_2 = \sigma_3$ ) at faialure in Figure 1. Average uniaxial compressive strength data from

Table 1: Number of triaxial tests at different temperatures, strain-rates and axial stress/confining pressure ratios  $(\sigma_r/\sigma_a)_*$ 

ξ	$\sigma_r/\sigma_a$	= 0.25	$\sigma_{r}/\sigma_{a}$	= 0.05	
T	10 <sup>-5</sup> /s	10 <sup>-3</sup> /s	10 <sup>-5</sup> /s	10 <sup>-3</sup> /s	
-5°C (23°F)	10 <b>V</b>		9V	9v	28V
-20°C (-4°F)		9V	9V	9v	270
	10V	9V	18V	18V	55V

V = Vertical

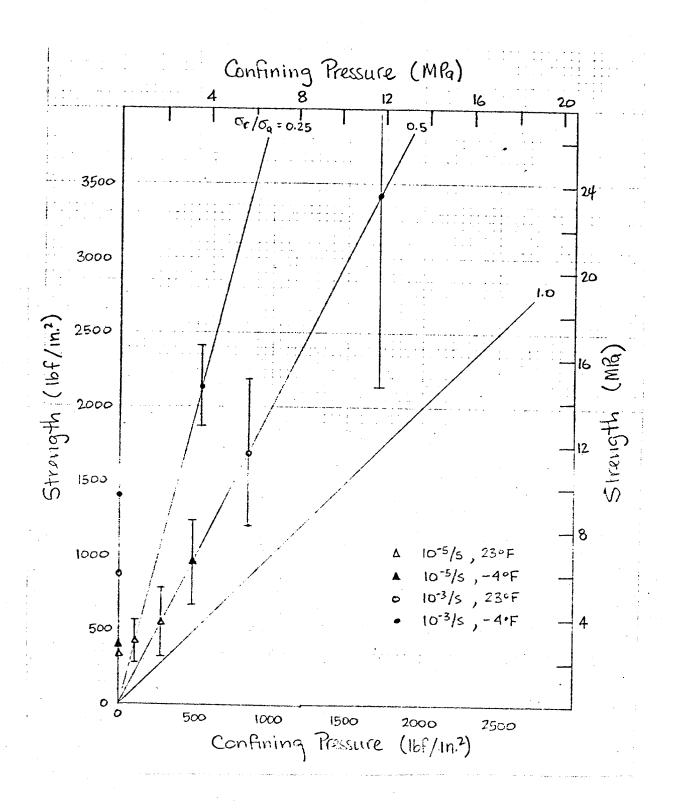


Figure 1: Compressive strength versus confining pressure for multiyear pressure ridge samples at different temperatures and strain-rates. The bars denote one standard deviation from the mean.

Phase I is included for comparison. In making comparisons between the unconfined and confined compressive strength data, it should be noted that the Phase I ridge samples had a much lower porosity. Table 2 summarizes the Phase II triaxial strength data.

As observed in Phase I, the confined compressive strength increases with decreasing temperature, increasing strain-rate, and increasing confining pressure. Due to variability of the ice structure between samples, the data show considerable scatter. The data at  $10^{-5}/s$  suggests that failure of the ridge ice samples at low strain-rates may be described by a Tresca or Von Mises yield criteria. The yield surface paralles the hydrostat ( $\sigma_{\mathbf{r}}/\sigma_{\mathbf{a}}=1.0$ ). This supports the observations made by Jones who investigated the confined compressive strength of fresh water polycrystalline ice at low strain-rates.

The Phase II final report will also include a discussion on the initial tangent modulus data from the triaxial tests and examine the variation of the triaxial strength and modulus data with sample porosity.

Table 2. Summary of triaxial strength data.

## Triaxial Strength

	(MPa)	ximum (lbf/in. <sup>2</sup> )	(MPa)	nimum (1bf/in.	. <sup>2</sup> ) (MPa) (MPa)	lean M 1bf/in. <sup>2</sup> )	lean Porosity (ppt)	Samples
-5°C (23°F)								
$10^{-5}/s$ V, 0.25	3.95	573	1.14	166	2.86±0.98	415±142	79	10
$10^{-5}/s$ V, 0.50	6.61	959	2.28	330	3.81±1.59	552±231	86	9
$10^{-3}/s$ V, 0.50	17.94	2602	5.43	788	11.70±3.41	1697±495	78	9
-20°C (-4°F)								
$10^{-3}/s$ V, 0.25	17.07	2475	11.58	1679	14.77±1.90	2141±275	77	9
$10^{-5}/s$ V, 0.50	11.03	1600	3.95	573	6.59±1.97	956±286	82	9
$10^{-3}/s$ V, 0.50	38.63	5602	8.34	1210	23.50±8.73	3408±1266	57	9

## APPENDIX A

## Triaxial Test Data

This appendix contains the results from the constant strain-rate triaxial tests (TRI). The parameters listed for each test are defined in the Index. As no displacement transducers were placed directly on the sample, the initial tangent modulus data given in Column 8 is based on the full sample strain. TRI-3-5/.5 denotes those tests conducted at a strain-rate of  $10^{-3}$ /s, a temperature of -5°C, and a confining pressure/axial stress ratio of 0.5.

Column No.	Symbol Symbol	Description
1	$\sigma_{m}$ , psi	Peak Stress
2	$\varepsilon_{\rm m}^{\rm m}({\rm GL})$ ,%	Strain at $\sigma_m$ determined by the DCDT's over a guage length of 5.5 inches
3	$\varepsilon_{\rm m}({\rm FS})$ ,%	Strain at $\sigma_m$ determined by the extensometer over the full sample length of 10 inches
4	t <sub>m</sub> , sec	Time to peak stress
5	m σ <sub>e</sub> , psi	Stress at end of test
6	e ε <sub>e</sub> (FS),%	Full sample strain at end of test
7	t, sec	Time to end of test
8	E <sub>i</sub> (GL),x10 <sup>6</sup> psi	Initial tangent modulus determined using strains found over the gauge length
9	E <sub>Q</sub> (GL),x10 <sup>6</sup> psi	Secant modulus determined using gauge length strains
10	E <sub>O</sub> (FS),x10 <sup>6</sup> psi	Secant modulus determined using full sample strains
11	S <sub>i</sub> ,°/00	Sample salinity at test temperature
12	ρ, 1b/ft <sup>3</sup>	Sample density at test temperature
13	V <sub>b</sub> ,°/00	Brine volume at test temperature
14	v <sub>a</sub> ,°/ <sub>00</sub>	Air volume at test temperature
1.5	n,°/00	Porosity at test temperature
16	σ <sub>e</sub> /σ <sub>m</sub>	Ratio of end to peak stress
17	Ice squareness, in	Sample squareness after ends are milled
18	End cap squareness, in	Sample squareness after endcaps are mounted
19	Shim, in	Amount of shim stock inserted between low end of sample and actuator before testing
20	vo	Initial Poisson's Ratio; circumferential and gauge length strain measurements used
21	v	Poisson's ratio at peak stress; $\epsilon_{m}( extsf{C})$ and $\epsilon_{m}( extsf{GL})$ used
22	$\varepsilon_{\mathrm{m}}^{\mathrm{m}}(\mathrm{C})$ ,%	Strain at $\sigma_m$ determined by the circumferential extensometer
1	1	

Also Note: FILE B-3-5 indicates Below Level Ice  $\frac{10^{-3}}{\text{sec Strain Rate}}$   $-5^{\circ}\text{C Test Temperature}$ 

iO
Ñ
r)
•
S
1
-
œ
Ë
ш
_
-
ш.

2 0.	SAMPLE # 02 03 04	ເ	90	67	67 68	60	10	11	12	13	14	5	16	17	09 10 11 12 13 14 15 16 17 18 19	19	
RA208-134/161	0.940 930.0	360 5.00	5 • 0 0	5360			0.049	0.049 0.03 51.12	51.12	რ ა	107.6	107.9	0.783	0.009	C.3 107.6 107.9 0.783 0.009 0.000 0.000	0000	
RA208-166/193	93 0•790 750•0 310 5•00	310	5.00	50.00 0.707	707.0	•	0.054	0.04 52.77	52.77	0 4	78.9	79.3	0.726	0.004	0.4 78.9 79.3 0.726 0.004 0.003 0.004	400	
RA208-1987225 366 0.920 940.0	0 349 0	326	326 5.00	5000 0.168	0.168		0 + 0 • 0	0.040 0.16 51.22	51.22	1.4	106.1	107.5	0.891	600.0	1.4 106.1 107.5 0.891 0.009 0.007 0.008	0.008	
RA208-259/280 515	0.580 590.0		334 5.00	5000 0.408	0 • 4 08	,	0.089	0.80 52.65	52.65	7.3	82.1	80 90 4	0.649	0.003	7.3 82.1 89.4 0.649 0.003 0.005 0.005	9000000	
RB212-077/104 166 0.360 342.0	3 342.0		165 5.00	5000 0.217	0.217		0.046	1.24	54.60	11.7	48 9	9 0 9	466.0	3.004	0.046 1.24 54.60 11.7 48.9 60.6 0.994 0.004 0.004 0.004	0.00	
88212-163/190 555 1.000 999.0	1 999.0		377, 5,00	5000 0.309	0.309		0.056	0.056 0.12 53.34	53 • 34	- -	69.1	70.2	629 0	0.008	1.1 69.1 70.2 0.579 0.008 0.002 0.003	200-6	
RB212-194/221 549 1.000 999.0	1 999.0		376 5.00	5000 0.245	0.245		0.055	0.33 53.75	53.75	3. 1.	62.3	65.3	0.685	600-0	3.1 62.3 65.3 0.685 0.009 0.009 0.010	0.00	
88213-066/093 286 0.450	0.450 444.0		230 5.00	5030 0.179	0.179	* .	9-064	0.064 1.03 54.05	54.05	9 6	58.1	67.7	9000	600.0	9.6 58.1 67.7 0.804 0.009 0.016 0.016	0.016	
RB213-097/124	0.065	201	5.00	5000 0.194	0.194		0.065	1.21	53.73	11.2	64.0	75.2	0.794	0.010	1.21 53.73 11.2 64.0 75.2 0.794 0.010 0.006 0.006	9000	
RA208-290/317 573 0.790 820.0 402 5.00	820.0	402	5.00	5000 0.291	1.291	-	7.0 7.3	9 62 0	4.10	7.4	9.	2 7 7	4		0-073 0-79 54-10 7-4 56-9 66-3 0-70 0-011 0-000		

FILE	FILE TRI-5-5/.5	5-5/4-5	2.9	80	ි න	0	11 12		14	13 14 15 16 17 18 19	16	17	18	19	20
2				0.172			0.09 47.23	1.0	0.7 175.6 176.3	176•3	6	.010	0.010 0.008 0.008	8000	
8A210-459/486 8446		442 5.00	១១១១១	0.267		.016	0.016 0.29 51.48	2.6	101.7	2.6 101.7 104.3 0.991 0.004 0.016 0.010	991 0	0 400	0.010 0	•010	
3,963,950.0		641 5.00	១១១ទ	0.297	0		0.093 0.51 55.76	4.9	27.4	4.9 27.4 32.3 0.716 0.010 0.010 0.010	716	0 010 0	)•010 0 0	010	
RB213-255/282 362 362	0 350	5 • 0 0	ខ្មួន	0.212	<b>ප</b>	• 056	0.056 2.22 55.12	21.1	41.2	21.1 41.2 62.3 0.967 0.004 0.004 0.004	1964	0.034	0 600 0	• 0 0 •	
0.739 730.0	0 430		2°00 2000	0.209		190.0	0.067 1.43 53.11 13.1 75.1 88.2 0.879 0.005 0.004 0.004	13.1	75.1	88.2 0	879 (	0.005	0 • 0 0 • 0	• 004	
0.918 913.0		330 5.00	5000	5800 0038	. 6	.038	0.038 1.15 54.69 10.8 47.2 58.0 0.943 0.006 0.006 0.006	10.8	47.2	58.0 0	943	900.0	0 900 0	900•	•
0.0000000000000000000000000000000000000	525	ა. მ		5903 0.287	C,	3.064	3.054 1.31 56.42 11.0 16.9 27.9 0.910 0.011 0.009 0.010	11.0	16.9	27.9 0	.910	0.011	0 600.0	.010	
0.520 540.0		625 5.00 5000 0.436	5003	0.436		0.184	0.184 1.04 56.62 10.1 15.0 25.2 0.652 0.034 0.014 0.014	10.1	15.0	25.2 0	.652	0.034	0.014.0	.014	
RA215-372/599 1.320 1340	+0 485	485 5.00 5000 0.339	5000	0.339		0.042	0.042 0.09 45.66		203.1	0.7 203.1 203.8 0.871 0.006 0.006 0.006	.871	900•0	0.006	900•	

21

	FILE	FILE TRI-3-5/.5	-5/.5														
SAMPLE # 02: 03	03 04		25 06	67	80	60	10	e≓ e=1 -	11 12	13	14	15	15 16 17	17	18	<u>σ</u> -	23
RA210-490/517	5.80	1369	0.596 5.80 1369 5.00 50.00		0.574	<b>(3</b>	0.310	0.32 53.58	3.58	ന • ജ	65.2	68.2	0.748	0-004	68.2 0.748 0.004 0.006 0.006	900*0	
RA211-233/260 0.440 4.20 979 5.00 50.00	.4.20	919	5.00 5		0.461	O	0.351	0.84 52.34	2.34	0 • 4	86.4	86.7	0.534	0.007	86.7 0.534 0.007 0.016 0.015	0.015	• .
RB213-286/313 1870 0.410 4.00 1432 5.00 50.00	6.0 • 4	1432	5.005		0.650	0	0.456	1.34 55.14		12.7	39.5		0.766	0.010	52.2 0.766 0.010 0.003 0.004	0 0 0 ° 0	
RB216-124/151 0.520 4.70 979 5.00 50.00	4.70	616	5.00 5		0.685	0	0.370	1.50 53.32		13.8	71.5	85.3	0.508	0.007	85.3 0.508 0.007 0.016 0.016	0.010	
RB216-262/289	3.10	1019	39 0.350 3.10 1019 5.00 50.00		0.526	0	0.370	0.95 51.61	1.61	8.5.1	4.00	108.9	0.554	0.012	8.5 100.4 108.9 0.554 0.012 0.013 0.014	0.014	
RB217-236/263 1638 0.420 3.90 1424 5.00 50.00	3.90	1424	5.00.5		0.601		0.438	0.40 53.13	3.13	2.7	73.2	76.8	0.775	0.005	3.7 73.2 76.8 0.775 0.005 0.006 0.006	990.0	
RB217-267/294 1584	5.60	1424	94 0.600 5.60 1424 5.00 50.00		0.531	0	0.264	0.95 55.13	55.13	9 • 3	39.0	48.1	0.899	0.019	9.3 39.0 48.1 0.899 0.019 0.008 0.008	0.008	
RB217-399/426 2602 1.010 10.60 1806 5.00 50.00	10.60	1806	5.00.5		0.715	, Co	0.258	0.62 56.51	6.51	<b>6</b> • 0	6.0 14.5	20.6	0.694	0.016	20.6 0.694 3.016 0.005 9.006	900*0	
RA210-059/086 0.280 2.80 788 5.00 50.00	2.80	788	5.00.5		0.419	0	0.281	0.02 48.38	18.38	ر د د	55.5	155.7	1.000	0.025	0.2 155.5 155.7 1.000 0.025 0.004 0.004	0.004	

AMPLE # 02 03	03 04 05	SO	90	67	. 80 20	60	10	=======================================	12	13	14	15	16	17	18	09 10 11 12 13 14 15 16 17 18 19 20	20	
A208-025/052 2125	6.00	1114	30 <b>•</b> s	2 .610 6.00 1114 5.00 50.00 0.482	0.482	, <b>C</b>	.348	C. 02 t	0.348 0.02 50.07 0.1 128.0 128.0 0.524 0.004 0.002 0.002	0.1	128.0	128.0	0.524	0.004	0.002	0.002		
(4258-346/367 .2467 0.700 7.20	7.20	995	5.00	995 5.05 50.00 0.334	0.334		.352	0.352 1.10 56.46	36 • 46	3.6	17.7	21.4	0.403	0.031	3.6 17.7 21.4 0.403 0.031 0.012 0.012	0.012		
(A211-078/105 1679 0.460 1679 0.46 4.60 0.428	4.60	1679	3.46	4.60	0.428	2	.365	0.365 0.02 50.95	50.95	0.1	0.1 112.6 112.7	112.7		900.0	0.006 0.002 0.002	0.002		
(A211-127/154 1822 0.470 4.80 740 5.00 50.0	08•	740	5.00	50 • 03	0.0 0 470	, 6	388	0.388 0.03 49.36	19.36	0.1	140.3	140.4	0.406	0.010	0.1 140.3 140.4 0.406 0.010 0.004 0.004	0.004		
18212-132/159 2475	6.440	1066	5.00	50.03	00 0.555	0	1.381	0.381 0.23 52.60	52.60	0.7	84.1	84.8	0.431	0.007	0.7 84.1 84.8 0.431 0.007 0.003 0.004	0.004		
(8212-326/353 (2157 0.610 6.30 1027 5.00 50.00 9.297	6.30	1027	5 0 B	50.00	0.297		.354	1.28	0.354 1.28 54.89 4.1 45.2 49.3 0.476 0.006 0.004 0.004	4•1	45.2	49.3	0.476	900•0	0.004	0.004		
(8212-047/074 1974 0.610 6.10	6.10	788	5.00	788 5.00 50.60 0.546	0.546	0	.324	0.31 5	0.324 0.81 53.36 2.5 71.4 73.9 0.399 0.003 0.006 0.006	2 • 5	71.4	73.9	0.399	0.003	900.0	900.0		
(8212-239/266 2236 0.58,0 5.70	5.70	931	931 5.00 50.	30 <b>•</b> 63	685.0 30	83	186.0	1.70 €	\$386.0 1.70 55.39 5.5 36.9 42.4 0.416 0.007 0.004 0.004	5	36.9	42.4	0.416	0.007	0.004	0.004		
18213-156/183 2332 0.690 6.90 1027 5.00 50.00 0.536	93	1027	5•00	50.00	0.536	0	.338	1.58	0.338 1.58 55.48 5.1 35.3 40.4 0.440 0.007 0.005 0.006	5.1	35.3	40.4	0.440	0.007	0.005	900*0		

50.4 0.910 0.008 0.005 0.006

5.9 44.5

1.83 54.96

0.095

5.00 5000 0.391

955

21

FILE TRI-5-20/.5

SAMPLE # 02 03 04 05 06 07	0.4	ស	0.6	5.2	08 69	10	08 09 10 11 12 13 14 15 16 17 18 19	13	14	15	6 17	18	19	50
RA210-194/221 2674 6.610 5.40 1942 5.00 50.00	ं ठ • 4	1942	0 0 • G		0.830	0.438	0.438 0.02 51.77 0.1 98.4 98.4 0.700 0.002 0.004 0.004	0.1	98.4	98.4 0.70	0 0 0 002	0.004.0	•004	
RA210-341/368 0.790 7.80 1958 5.60 50.00	7.80	1958	5.00		0.689	0.326	0.326 0.11 51.48	0.3	103.5	0.3 103.5 103.8 0.760 0.011 0.003 0.004	0 0.011	0.003 0	*00 <b>*</b>	
RA210-567/594 4011	8.50	2196	5.00	50.03	1.321	0.456	0.456 0.83 56.43 2.7 18.0 20.7 0.547 0.007 0.003 0.004	2.7	18.0	20.7 0.54	7 0.007	0.003 0	•00•	
RB213-225/252 3608 9-680 6-60 1448 5-00 50-00	6+50	1448	5.00	50.00	106.0	0.442	0.442 2.22 55.25 7.2 39.9 47.0 0.481 0.004 0.004 0.010	7.2	39.9	47.0 0.48	1 0.004	0.00.0	.010	
R8213-342/369 4584 1.000 9.40 2992 5.00 50.00	9.40	2992	5.00		0.936	0.458	0.458 1.65 55.60 5.4 33.2 38.6 0.653 0.004 0.004 0.004	5.4	33.2	38.5 0.65	3 0.004	0.004 0	•004	
RB216-089/116 3374 0.490 4.50 3374 0.49 4.50	4.50	3374	0.49		0.838	0.689	0.689 0.29 52.55	0.9	0.9 85.0 89.9	89.9	0.006	0.006 0.004 0.004	<b>•00</b>	
86216-392/419 3629 3629	90.6	1974	5.09		0.860	0.399	0.399 1.78 56.22 5.8 22.6 28.4 0.544 0.007 0.010 0.010	5.8	22.6	28.4 0.54	4 0.007	0.010.0	.010	
R3217-052/079 0.210 2.00 1210 0.21 2.00	2.00	1210	0.21		0,639	0.576	0.576 0.14 53.71 0.4 64.7 65.1	0.4	64.7	65.1	0.007	0.007 0.002 0.002	-002	
88218-363/390 5662 1.205 12.00 2929 5.60 50.00	12.00	2929	5.00	50.00 1	1.000	0.467	0.467 0.55 56.60 1.8 14.8 16.6 0.523 0.005 0.005 0.006	1.8	14.8	16.6 0.52	3 0.005	0.005 0	900•	

and the control of the state of

# MECHANICAL PROPERTIES OF MULTI-YEAR SEA ICE, PHASE II

Progress Report 1

March 1983

Ъу

G.F.N. Cox, J.A. Richter, W.F. Weeks and M. Mellor

Prepared for
Shell Development Company
Minerals Management Service

## INTRODUCTION

This progress report describes the work performed in the Mechanical Properties of Multi-Year Sea Ice Program, Phase II, from 14 April 1982 to 31 January 1983. During this period, ice samples were obtained from several multi-year pressure ridges in the Beaufort Sea and shipped to CRREL for testing. Uniaxial compression tests were then performed on both horizontal and vertical samples to examine the effect of orientation on ice strength. We also completed the uniaxial tension and constant load compression tests described in the proposal. During the latter part of January we began working on the constant load tension and triaxial tests.

This progress report includes a detailed description of the field sampling program and presents the results from the constant strain-rate compression and tension tests. The next progress report will include the results from the triaxial tests.

#### FIELD SAMPLING PROGRAM

Our Phase I sampling program occurred between 3 and 15 April 1981.

Although we had scheduled the Phase II sampling program for essentially the same time period in 1982, delays associated with establishing the funding level of the project kept the field operation from starting until 14 April 1982. Therefore, we were very concerned that a period of "warm" weather would cause us difficulties and that we would have to load the shipping boxes with dry ice while at the sampling sites. Fortunately this did not occur. Also we were favored with extremely good flying and working weather (limited ice fog and light winds). In fact the one day that ice fog curtailed flying we had not planned to work on the ice as a dry ice shipment

We finally selected a floe north of Leavitt Island where several floes were located near to each other and contained several well-rounded ridges that we estimated to be at least two summers old (Figure 1). The first sampling location (Ridge A) was on a thick multi-year floe with lateral dimensions of roughly 50 m. Although the ice had clearly been deformed, there was no cleanly delineated linear ridge. Therefore, we sited two of our sampling locations on high points and two sampling locations in swales. A sketch map showing the general topography of the sampling area is given in Figure 2. Figure 3 shows an oblique aerial photograph of the site (located in the foreground). The small building (which was transported to the site by helicopter) provides a sense of scale. The location of Hole 10 (see Figure 2) is indicated by an arrow in Figure 3). Figure 4 shows a surface view of the floe. The 1 to 1.5 m freeboard is evident. A total of 11 holes were taken at this site for a total core length of 48.70 m. The ice at this site was generally characterized by a high volume of included air as compared with the ridges that we had sampled in 1981. Therefore, we decided to sample several other ridges in the near vicinity to see if they also contained such large amounts of included gas or perhaps would prove to be similar to the ridges we sampled previously.

These additional ridges were found on two floes located approximately 200 m to the north of our first sampling site. The second ridge (Ridge B) was approximately 27 m long and was located on the smallest of these two floes (Figure 5). It is possible that these two floes were initially part of the same larger floe, which had been split. The ice proved to be quite solid

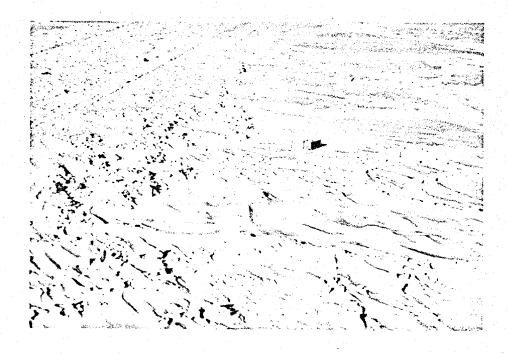


Figure 1: Aerial view of multi-year floe, designated as Ridge A, where first 11 cores were obtained.

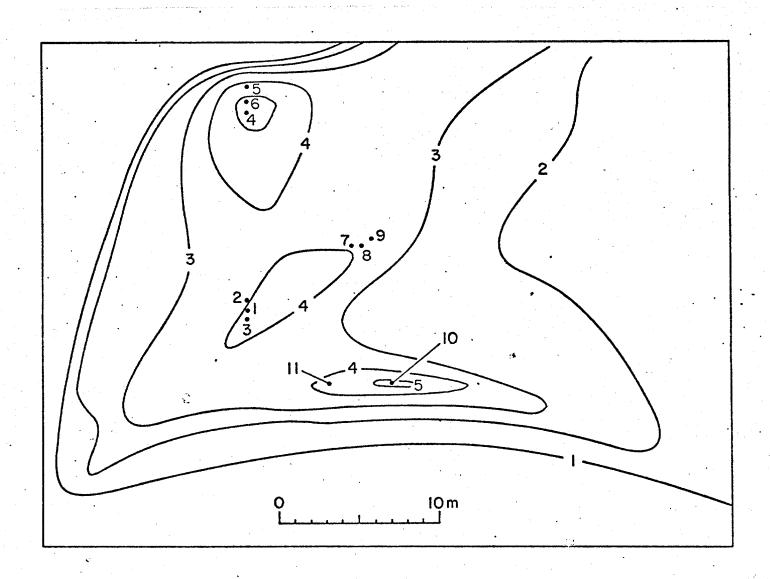


Figure 2: Sketch map of multi-year floe, Ridge A, showing location of the ice sampling sites.

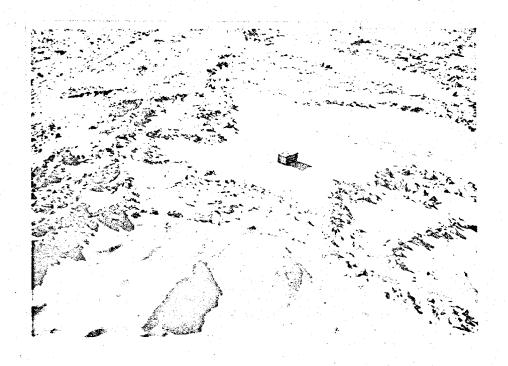


Figure 3: Oblique aerial view of Ridge A sampling site.



Figure 4: Surface view of Ridge A sampling site.

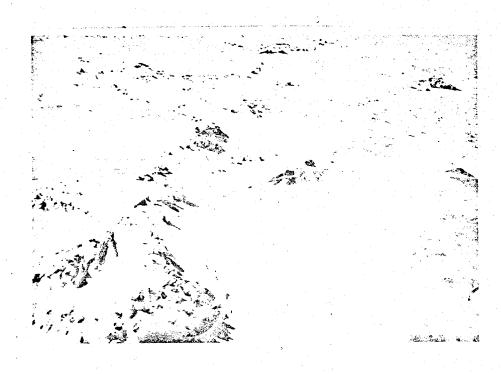


Figure 5: Aerial view of sampling area containing Ridges B, C, and D.

and massive with significantly less included gas. A sketch of a profile of this ridge showing the location of specific core sites is given in Figure 6. Note the sharp vertical termination of the ridge on the "right-hand" edge of the floe. The length of core obtained from this ridge was 50.32 m.

The third ridge sampled was approximately 75 m long and was the largest ridge on the adjoining floe. A profile of this ridge (Ridge C) is given in Figure 7. Figure 5 shows an aerial view of this ridge (marked as C) as well as of Ridges B and D. Ridge C, although broad, was quite clearly defined. Figure 8 shows coring underway on this ridge. A total of 67.11 m of core were obtained from this ridge for use as test specimens. A 9.53 m core was also obtained (penetrating completely through the floe) to use in petrographic studies. In addition all the 12 in. diameter core was obtained from this ridge.

The last ridge sampled was roughly parallel to Ridge C. Ridge D can also be seen in Figures 5. This ridge was 53.6 m in length and also was clearly delineated. Figure 9 shows the split end of the ridge where its blocky deformed structure could be examined. The total core recovery from Ridge D was 47.93 m.

In addition, 3.83 m of core were obtained from a floe which appeared to contain undeformed multi-year ice. Figure 10 shows an aerial view of the site. A judgment of the adequacy of this selection will await petrographic examination of the core.

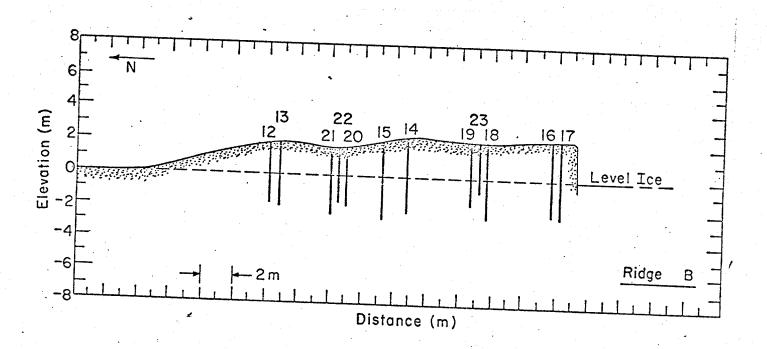


Figure 6: Sketch of Ridge B profile showing the location of the ice sampling sites.

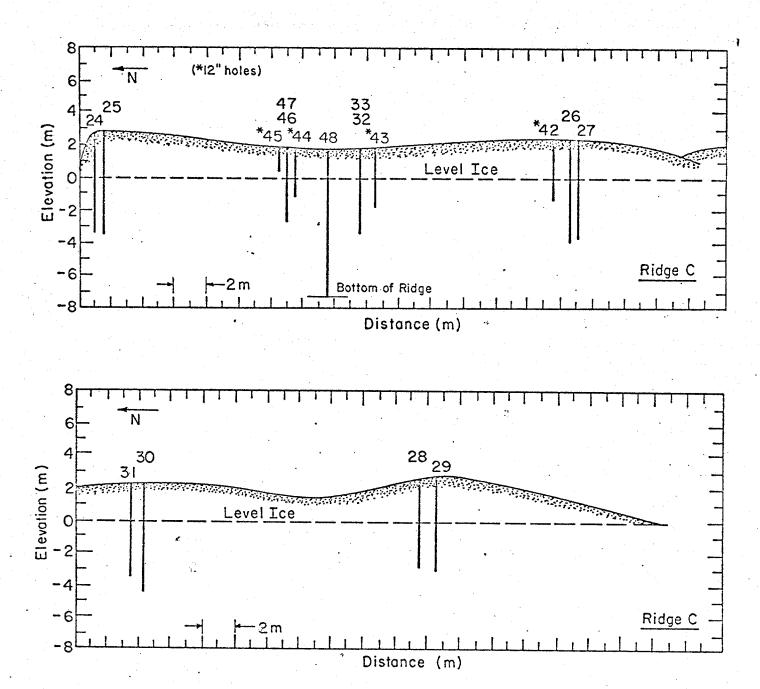


Figure 7: Sketch of Ridge C profile showing the location of the ice sampling sites.

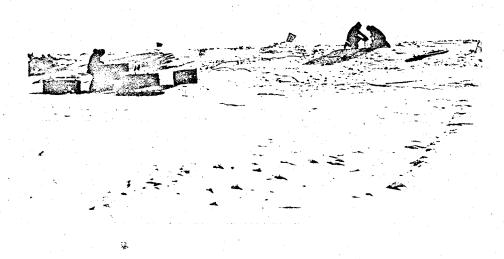
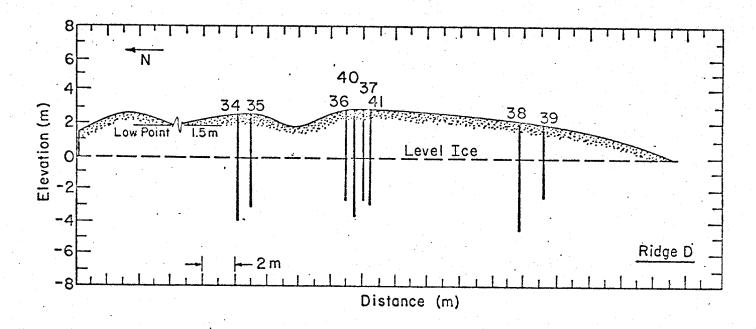


Figure 8: Coring operation on Ridge C.



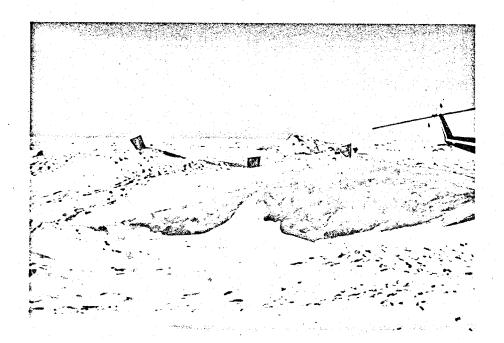


Figure 9: Split end portion of Ridge D.

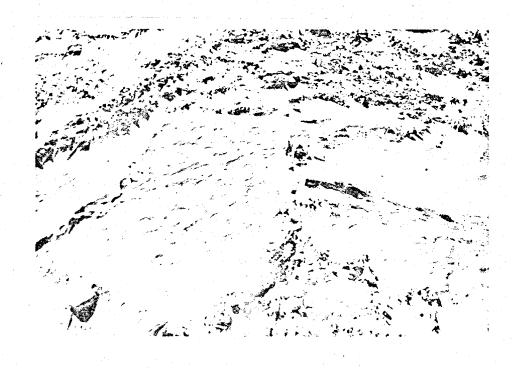


Figure 10: Aerial view of multi-year floe where undeformed samples were obtained.

Table 1 summarizes the estimated height of the top of each core hole above level ice (approximate sea level). Also given is the penetration depth (the total length cored from each hole). Table 2 gives the daily drilling log and Table 3 provides a summary of this data. The primary part of the coring program was carried out with the 4-1/4 in. corer in four days (15-18 April) with a total of 205.6 m of core recovered. The total number of samples (vertical) obtained from this core were 439 or roughly 100 samples per day. The total length of 12 in. diameter core obtained was 12.79 m which resulted in 61 horizontal specimens giving a grand total of 500 specimens for the season. As mentioned we also obtained 8.75 m of core for petrographic studies.

### Core Logging Procedures

There were some differences in the core logging procedures between the 1981 and 1982 field seasons. These were the result of two different factors. First, as the result of the Phase I testing program, we had found that some of our field measurements did not prove to be particularly useful. For instance in 1981 we took rather detailed temperature and salinity profiles in the field. As the important temperature is the ice temperature at the time of testing, in 1982 we reduced the temperature measurements in the field to three or four per core, a number sufficient to indicate the general temperature profile in the ice. We also reduced the number of subsidiary salinity measurements as we have not found brine drainage to be an important problem in the low salinity multiyear ice we have been sampling. Also our routine laboratory procedures include a salinity determination on

Table 1. 1982 Ridge Heights and Penetration Depths.

Location			Height	Depth	Height	Depth	Dia.	Domonika
R82A	15 Apr 82	1	230 cm	461 cm	7.7 ft	15.0 ft	$4.\overline{25}$ in	Remarks
		2	234 cm	384 cm	7.7 ft	12.6 ft		
•		3	234 cm	473 cm	7.7 ft	14.3 ft		
		4	300 cm	581 cm	9.8 ft	19.1 ft		
		5	345 cm	454 cm	11.3 ft	14.9 ft		
		6	300 cm	502 cm	9.8 ft	16.5 ft		
		7	234 cm	373 cm	7.7 ft	12.2 ft		
		8	234 cm	373 cm	7.7 ft	12.2 ft		•
		9	234 cm	377 cm	7.7 ft	12.4 ft		
		10	503 cm	601 cm	16.5 ft	19.7 ft	4.25 in.	
		11	406 cm	327 cm	13.3 ft	10.7 ft	4.25 in.	
R82B	16 Apr 82	12	203 cm	380 cm	6.7 ft	12.5 ft	4.25 in.	
		13	203 cm	409 cm	6.7 ft	13.4 ft	4.25 in.	
*	•	14	249 cm	472 cm	8.2 ft	15.5 ft	4.25 in.	
		15	218 cm	479 cm	7.2 ft	15.7 ft	4.25 in.	
		16	249 cm	473 cm	8.2 ft	15.5 ft	4.25 in.	
		17	249 cm	482 cm	8.2 ft	15.8 ft	4.25 in.	
		18	234 cm	473 cm	7.7 ft	15.5 ft	4.25 in.	
	•	19	234 cm	396 cm	7.7 ft	13.0 ft	4.25 in.	
•		20	185 cm	361 cm	6.1 ft	11.8 ft	4.25 in.	
		21	185 cm	427 cm	6.1 ft	14.0 ft	4.25 in.	•
		22	185 cm	354 cm	6.1 ft	11.6 ft	4.25 in.	
		23	234 cm	326 cm	7.7 ft	10.7 ft	4.25 in.	
R82C	17 Apr 82	24	269 cm	624 cm	8.8 ft	20.5. ft	4.25 in.	
		25	269 cm	639 cm	8.8 ft	21.0 ft	4.25 in.	
		26	234 cm	652 cm	7.7 ft	21.4 ft	4.25 in.	
		27	234 cm	544 cm	7.7 ft	17.8 ft	4.25 in.	
		28	269 cm	565 cm	8.8 ft	18.5 ft	4.25 in.	
		29	269 cm	558 cm	8.8 ft	18.3 ft	4.25 in.	
		30	221 cm	680 cm	7.3 ft	22.3 ft	4.25 in.	
		31	221 cm	576 cm	7.3 ft	18.9 ft	4.25 in.	
		32	173 cm	563 cm	5.7 ft	18.5 ft		SL163cm/5.3ft
R82D	10 4 00	33	173 cm	470 cm	5.7 ft	15.4 ft	4.25 in.	OD1410cm/46.3ft
KOZD	18 Apr 82	34	269 cm	676 cm	8.8 ft	22.2 ft	4.25 in.	
	. •	35	269 cm	564 cm	8.8 ft	18.5 ft	4.25 in.	
	•	36	300 cm	567 cm	9.8 ft	18.6 ft	4.25 in.	
	•	37	300 cm	577 cm	9.8 ft	18.9 ft	4.25 in.	
•		38	218 cm	682 cm	7.2 ft	22.4 ft	4.25 in.	
		39 40	218 cm	466 cm	7.2 ft	15.3 ft	4.25 in.	
		41	300 cm	678 cm	9.8 ft	22.2 ft	4.25 in.	
R82C	19 Apr 82	42	300 cm	583 cm	9.8 ft	19.1 ft.	4.25 in.	
R82C	20 Apr 82	43	234 cm	404 cm	7.7 ft	13.3 ft	12 in.	
	20 Hpl : 02	44	173 cm	389 cm	5.7 ft	12.8 ft	12 in.	SL163cm/5.3ft
		45	173 cm 173 cm	323 cm	5.7 ft	10.6 ft	12 in.	OD1410cm/46.3ft
		46	173 cm 173 cm	163 cm	5.7 ft	5.3 ft	12 in.	II II
		47	173 cm	364 cm	5.7 ft	11.9 ft	4.25 in.	н
R82C	22 Apr 82	48	173 cm	476 cm	5.7 ft	15.6 ft	4.25 in.	
R82E	22 Apr 82	49	30 cm	953 cm 383 cm	5.7 ft	31.3 ft	4.25 in.	11
	-t- ~=		20 CIII	JOJ CIII	1.0 ft	12.6 ft		SL53cm/1.7ft
								OD1920cm/63ft

<sup>\*</sup>SL = Sea level below top of hole OD = Ocean depth

	Ridge	"			Total
Date		Hole #	Drill Used		Depth (m)
4/15/82	R82-A	1	Blue 4-1/4	122, 108, 100, 94, 37	4.62
		2	**	122, 112, 97, 53	3.84
		3	•	119, 115, 98, 66, 39	4.37
		4		128, 102, 102, 94, 100, 55	5.81
		5		122, 93, 91, 102, 46	4.54
		6	11	126, 104, 100, 96, 76	5.02
		7	• <b>11</b>	121, 112, 95, 45	3.73
		8	et a series and a	127, 103, 105, 38	3.73
		9	tt	125, 106, 99, 47	3.77
	•	10	Ħ	127, 101, 105, 92, 105, 71	6.01
		11	11	74, 48, 98, 107	3.27
4/16/82	R82-B	12	Blue 4-1/4	127, 105, 56, 32, 40	3.80
	•	13	11	130, 91, 108, 80	4.09
		14	TT .	115, 111, 104, 96, 46	4.72
•		15	11	118, 107, 106, 98, 50	4.79
		16	11	119, 105, 102, 101, 46	4.73
		17	11	118, 114, 98, 106, 46	4.82
		18	•	121, 105, 110, 92, 45	4.73
		19	tt	110, 111, 102, 73	3.96
		20	Ħ	122, 104, 98, 37	3.61
		21	11	121, 116, 103, 87	4.27
		22		126, 107, 96, 24	3.54
		23	**	121, 106, 99	3.26
4/17/99	R82-C	24	Orange 4-1/4	105, 113, 100, 106, 102, 98	6.24
4/17/82	KOZ-C	25	orange 4-1/4	128, 102, 96, 101, 99, 113	6.39
		26	**	120, 114, 106, 114, 96, 102	6.52
		27	10		5.44
		28	11	120, 126, 120, 80, 98	5.65
•			14	117, 126, 124, 100, 98	5.58
		29	11	106, 109, 121, 116, 106	
		30	•	114, 123, 100, 123, 110, 110	6.80 5.76
		31	••	121, 108, 115, 112, 120	5.76
		32	•	110, 110, 116, 110, 117	5.63
1.410.400	700 P	33		126, 107, 121, 116	4.70
4/18/82	R82-D	34	Orange 4-1/4	114, 119, 111, 106, 123, 103	6.76
		35	11	115, 110, 109, 120, 110	5.64
		36	11	110, 117, 122, 113, 105	5.67
		37	**	116, 112, 125, 114, 110	5.77
		38	11	122, 111, 122, 113, 120, 94	6.82
		39	••	121, 112, 112, 121	4.66
		40	•	120, 123, 112, 124, 89, 110	6.78
		41	**	120, 114, 122, 124, 103	5.83
4/19/82	R82-C	42	Blue 12"	96, 82, 100, 60, 66	4.04
4/20/82	R82-C	43	Blue 12"	103, 80, 88, 52, 66	3.89
		44	••	94, 102, 71, 56	3.23
		. 45		101, 62	1.63
		46	Orange 4-1/4	120, 122, 122	3.64
		47	•	121, 117, 124, 114	4.76
4/21/82	No Drilling		Winds Blowin		
4/22/82	R82-C	.48	Orange 4-1/4	112, 116, 124, 113, 112, 102,	
				46, 58, 103, 67*	9.53
	R82-E	49	Orange 4-1/4	114, 30, 100, 68, 66*	3.83

<sup>\*</sup>Denotes bottom of pressure ridge.

Table 3. Summary of Daily Drilling.

Date	<u>Drill</u>	# Holes	# Cores	Average Core Length	Total Length of Core Obtained
4/15/82	Blue 4-1/4	11	52	93 cm	48.7 m
4/16/82	Blue 4-1/4	12	53	95 cm	50.32 m
4/17/82	Orange 4-1/4	10	53	110 cm	58.71 m
4/18/82	Orange 4-1/4	8	42	114 cm	47.93 m
4/19/82	Blue 12 in.	1	5	81 cm	4.04 m
4/20/82	Blue 12 in.	3	11	79 cm	8.75 m
	Orange 4-1/4	2	7	120 cm	8.4 m
4/22/82	Orange/Blue 4-1/4	2	13	92 cm	13.36 m

Total Length of 4 in. diameter core obtained - 226.09 m

Total length of 12 in. diameter core obtained - 12.79 m

Longest 4 in. diameter core obtained - 128 cm; hole no. 25

Longest 12 in. diameter core obtained - 103 cm; hole no. 43

each test specimen after testing is complete. It was decided that the number of salinity determinations was more than adequate.

In 1981 we shipped large quantities of extra core back to CRREL for use in petrographic studies. Much of this core had been damaged during extraction when the extended core dogs scored the core sides instead of cleanly breaking the core off at the bottom of the hole. Such core could not be used for test specimens. In 1982 this problem was resolved and very little damaged core was obtained. We also had found that we were able to save sufficient ice from each 33 cm rough test specimen as collected in the field to prepare the necessary subsidiary thin sections. Therefore, it was not necessary to ship extra-long test specimens or to include extra ice from each core. Samples were cut to 33 cm in the field and extra ice was not included. This resulted in a great saving in time and in shipping costs.

### Shipping and Storage of Ice Samples

Initially we had planned to use shipping and storage procedures identical with those that were used in 1981. However, because of the delay in going into the field we were not able to arrange for refrigerated storage at Anchorage (the majority of the refrigerated space is owned by fishing companies and the fishing season had started). In retrospect this problem did us a favor as it forced us to change to direct shipping from Deadhorse to Boston, a procedure that was both easier and successful.

Upon bringing the samples in from the field they were stored at ambient temperatures for a maximum period of six days. As the local temperatures were cold, no problems were experienced with brine drainage. The only additional procedure that was necessary was to pack the end of each core tube with paper to prevent damage to the cores while in transit. Six core tubes were sent in each packing box which was charged with 75 1bs of dry ice (the actual weight was somewhat less than this as there was some sublimation of CO2 during the shipment of dry ice to use from Anchorage). We then shipped the boxes via AIA with consignment to Emery Air Freight in Anchorage. Before the shipment left Deadhorse, Emery was able to reserve space on a Flying Tigers flight to Boston. Emery also handled the transfers between airlines in Anchorage as well as all the additional paper work. It proved to be a very efficient and satisfactory operation. USACRREL personnel finally met the Tigers flight in Boston with a refrigerated truck and transported the ice to Hanover. Spot checks of temperatures within the boxes upon their arrival in Hanover indicated suitably low temperatures (even in one box where the insulation had been pierced by a fork lift). We plan to repeat these procedures during any future sampling operations.

### Coring Procedures

Much of the success of the field program should be credited to the efficiency with which our coring equipment obtained the samples. The 4-in. diameter coring augers were, in fact, the same augers that were used in 1981. As mentioned, at that time we experienced difficulty with the core dogs gripping the sample firmly and producing a clean break at the base of

the core. Instead the dogs frequently formed long gouges in the sides of the samples. These gouges were of sufficient depth that such gouged ice could not be used as test specimens. During the 1982 field season this problem was resolved. A newly designed core dog which provided a more aggressive cutting edge replaced last years dogs. Secondly, an inverted impact hammer was designed to give the extension rods a sharp upward impact. This impact both seated the dogs and caused the core to break cleanly. Figure 11 shows the impact hammer in use. A third change was made to the four-inch coring system which included the addition of a short length of helical flighting directly above the augers. This kept the snow from packing into the top of the core and resulted in decreased friction when the core barrel is being removed from the hole. The helical flighting can be seen in Figure 12 and it is believed that this attachment allowed the drillers to obtain longer cores ranging from 100 cm to a maximum 128 cm.

The major new addition to the coring equipment was the development of a 12 in. diameter coring auger system. The auger itself was designed to obtain 12 in. diameter samples up to 1 m in length. The drill, simply stated, is an exploded version of the 4 in. auger. The barrel material is a reinforced fiberglass epoxy pipe. The cutters are attached to an aluminum disc at the bottom of the drill. The auger flights are nylon epoxed webbing. The top drive adapter couples the auger extensions to the auger. Figure 13 shows the auger attached to the winch and drive system. A commercially available gasoline operated post hole digger was modified to provide the rotation and lifting requirements to operate the drill. Figure 14 shows the mobile drill frame winching itself up a pressure ridge. Once the drill

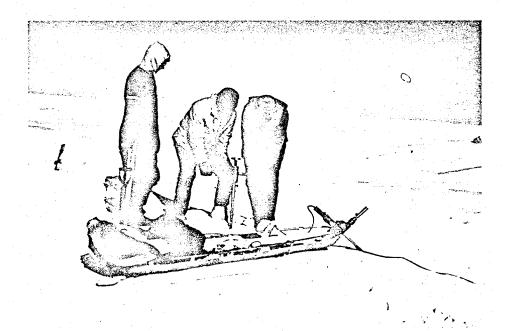


Figure 11: Impact hammer used to engage core dogs and break core.

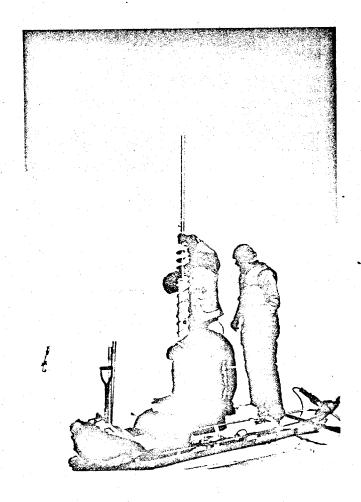


Figure 12: Helical flight on top of core barrel to prevent packing of cuttings above core barrel.

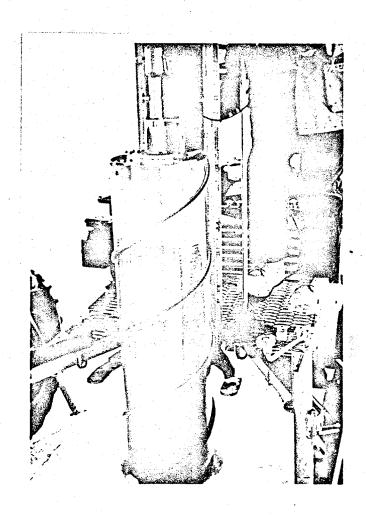


Figure 13: Twelve-inch diameter core barrel.

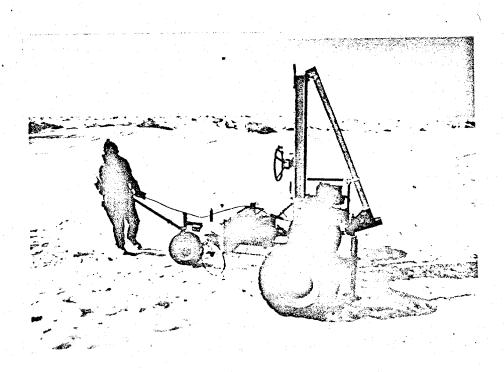


Figure 14: Mobile drilling rig used to support and drive the 12-inch diameter core barrel.

has augered approximately 1 m the drill is removed from the hole. A core retrieval system, Figure 15, is lowered into the hole. A horizontally mounted hydraulic cylinder at the top of this system is activated to shear the core at the bottom. Two core dogs located at the bottom of this device retain the core as it is lifted to the surface. The core is then removed from the retrieval system (Figure 16) and placed in a (log) carrier to move the core to the horizontal sampling drill. To obtain horizontal samples a simple drill press was designed such that 4 in. diameter cores could be obtained from the 12 in. core. Figure 17 shows this system in operation. The only problem encountered with the complete 12 in. system was caused by vibrations when using the horizontal sampling drill. This will be easy to correct in future models by simply adding additional stiffening elements to the drill frame.

to the second of the second of

The entire 12 in. drilling system was transported to the drilling site by sling loading the mobile frame under the helicopter. Once on the ground a winching system allows the operators to maneuver the system to the exact location desired for drilling. The drilling log is summarized in Table 3.

#### UNIAXIAL COMPRESSION TESTS

#### Sampling Scheme and Test Variables

Sixty-two constant strain-rate uniaxial compression tests were performed in Phase II. The tests were conducted at two strain-rates,  $10^{-4}/s$  and  $10^{-2}/s$ , and two temperatures,  $-20^{\circ}$ C and  $-5^{\circ}$ C, to supplement the tests performed in Phase I. In Phase I the compression tests were conducted at strain-rates

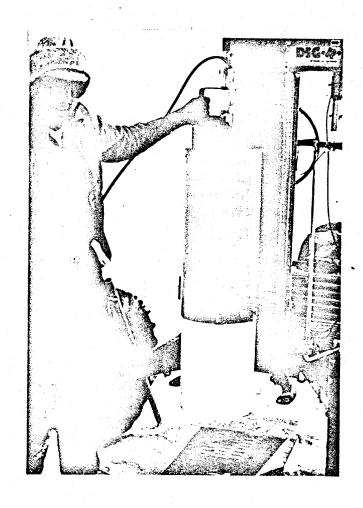


Figure 15: Core catcher used to break and retrieve 12-inch diameter core.

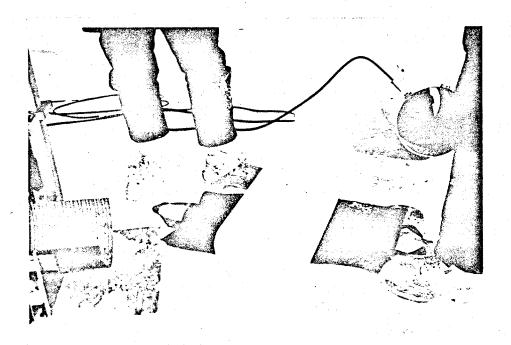


Figure 16: Removing 12-inch diameter core from core catcher.

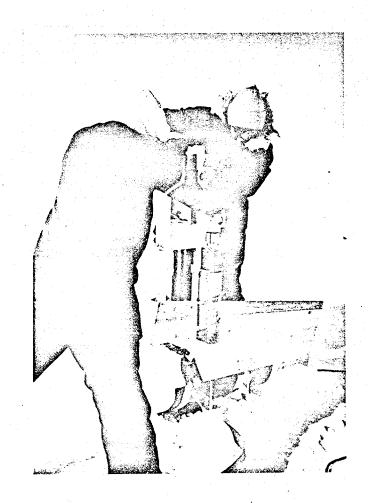


Figure 17: Drill press used to obtain horizontal samples from the 12-inch diameter core.

of  $10^{-5}$ /s and  $10^{-3}$ /s and at temperatures of  $-20^{\circ}$ C and  $-5^{\circ}$ C. Unlike Phase I, tests in Phase II were performed on both horizontal and vertical samples to assess the effect of sample orientation on ice strength. The number of tests at each test condition is summarized in Table 4. Details on the sample preparation and testing techniques are given in Mellor et al. (1983). The procedure used in Phase II were identical to those used in Phase I.

#### Uniaxial Compressive Strength

A detailed tabulation of the results from the constant strain-rate, uni-axial compression tests is given in Appendix A. The average compressive strength of the ice is plotted against strain-rate in Figure 18 and 19. Figure 18 contains the results from those tests conducted at  $-5^{\circ}$ C (23°F), and Figure 19 presents the results from those tests conducted at  $-20^{\circ}$ C ( $-4^{\circ}$ F). The bars denote one standard deviation from the mean. The test results from Phase I at  $10^{-5}$  and  $10^{-3}$ /s are also included for comparison. Average strength values from Phases I and II are listed in Table 5.

At a given temperature and strain-rate, the Phase II strength data show considerable scatter. Preliminary analyses in Phase I have demonstrated that the large variation in strength can be explained by large variations in the ice structure and porosity.

Based on our understanding of the variation of ice strength with strainrate we would expect a power law relationship between ice strength and strain-rate in the ductile range (Weeks and Mellor, 1983). On log-log

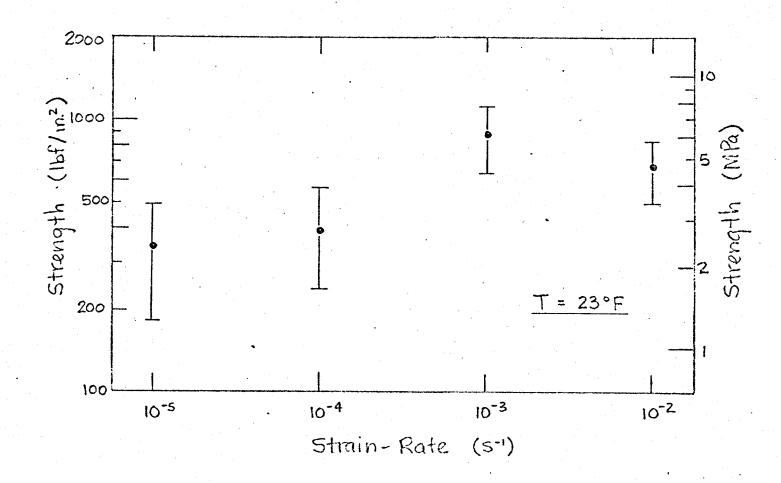


Figure 18: Uniaxial compressive strength versus strain-rate for samples tested at -5°C (23°F). The bars denote one standard deviation.

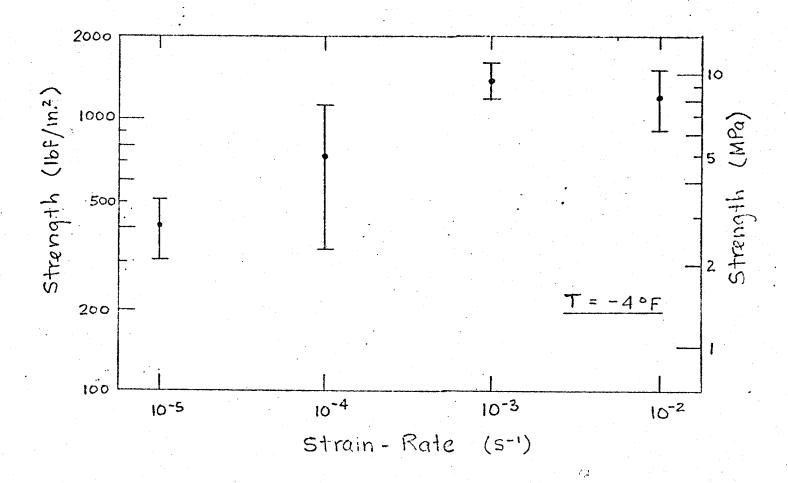


Figure 19: Uniaxial compressive strength versus strain-rate for samples tested at -20°C (-4°F). The bars denote one standard deviation.

Table 4: Number of uniaxial compression tests at different temperatures and strain-rates.

Ē			
T	10 <sup>-4</sup> /s	10 <sup>-2</sup> /s	
-5°C (23°F)	9 <b>v</b>	9v	18V
	10H		10H
-20°C (-4°F)	13V	9V ·	22V
	12H		12H
	22V	18V	40V
	22H		22H

6,2

Table 5: Summary of compressive strength data for Phases I and II.

# Uniaxial Compressive Strength

Maximum		Minimum		Me		Mean Porosity		
	(MPa)	(1bf/in. <sup>2</sup> )	(MPa)	(1bf/in. <sup>2</sup> )	(MPa)	(1bf/in.2)	(ppt)	<u>Samples</u>
-5°C (23°F)		•			•			•
		•						
10 <sup>-5</sup> /s V	7.52	1090	0.47	68	2.34±1.08	340±157	44	71
$10^{-4}/s V$	5.52	800	1.87	271	3.07±1.23	445±179	69	9
10 <sup>-4</sup> /s H	3.87	561	1.21	175	2.35±0.74	341±108	78	10
$10^{-4}/s$ all	5.52	800	1.21	175	2.69±1.04	390±151	73	19
	10.90	1580	2.39	346	6.06±1.63	879±237	46	69
$10^{-2}/s \ V$	6.42	931	2.69	390	4.67±1.17	677±169	68	9
-20°C (-4°F)	)	,						
	<u>~</u>							
$10^{-5}/s$ V	4.26	617	1.17	170	2.79±0.69	404±100	36	41
	12.73	1846	3.34	485	6.17±3.10	894±450	50	13
	7.02	1018	1.68	243	3.74±1.67	543±242	33	12
4.	12.73	1846	1.68	243	5.00±2.70	725±392	42	25
A .	12.68	1838	7.03	1020	9.63±1.39	1396±202	39	41
	10.48	1520	4.12	597	8.24±2.05	1195±297	74	9

H - Horizontal

paper, strength versus strain-rate would plot as a straight line. The combined average test results of Phases I and II at -5°C (23°F) do not show this tendency. The average strength of the  $10^{-4}$ /s tests is lower than anticipated. However, at -20°C (-4°F) the  $10^{-4}$ /s Phase II test average is in reasonable agreement with the  $10^{-5}$ /s and  $10^{-3}$ /s averages obtained in Phase I.

Since the strength of sea ice decreases with increasing porosity, it appears that the above observations can be explained in terms of the average ice porosity of the samples tested at each strain-rate and temperature. In Table 5 mean porosities are given for the samples tested at each test condition. It can be seen that at  $-5^{\circ}$ C, the  $10^{-4}$ /s tests have a much higher porosity than the tests conducted at  $10^{-5}$ /s and  $10^{-3}$ /s. At  $-20^{\circ}$ C, the mean porosities of the  $10^{-5}$ /s,  $10^{-4}$ /s, and  $10^{-3}$ /s are similar and the average strength values do show a power law relationship.

In both the -5 and -20°C tests conducted at a strain-rate of  $10^{-2}/s$ , there is an apparent decrease in ice strength relative to the tests conducted at  $10^{-3}/s$ . We attribute this decrease in strength to the much larger porosity of the  $10^{-2}/s$  samples.

Noticeable differences in strength were also found between the horizontal and vertical samples tested at  $10^{-4}$ /s. Generally, the vertical samples had a higher strength. At -5°C (23°F) the average strength of the vertical samples was 30% higher. At -20°C (-4°F) the average strength of the vertical and horizontal samples differred by 65%. While the -5°C (23°F)

vertical samples had a lower porosity, the -20°C (-4°F) vertical samples had a significantly higher porosity. However, the -20°C (-4°F) vertical samples contained two relatively strong specimens (Figure 23) which are believed to be columnar ice samples oriented in the hard fail direction.

The compressive strength of the samples is plotted against the total porosity of the ice in Figures 20 through 23. The air and brine volume equations given in Cox and Weeks (1982) were used to calculate the ice porosity from the ice salinity, temperature, and density. As in Phase I there is a tendency for the ice strength to decrease with increasing porosity. This trend is again most pronounced at high strain-rates,  $10^{-2}/s$ , where flaws and cavities play a more important role in brittle ice behaviour.

Thin sections have been prepared of the horizontal and vertical samples to examine the influence of ice structure on the ice mechanical properties.

This analysis should be completed at the end of May and will be included in the draft final report.

#### Residual Compressive Strength

The uniaxial compression tests on the testing machine were programmed to continue to 5% full sample axial strain to examine the residual strength and post-yield behaviour of the ice. The residual strength is defined as the stress on the sample at 5% strain assuming a constant 10.16 cm (4.000 in.) diameter specimen. Average values of the residual/maximum strength ratio of the ice samples under different loading conditions are given in Table 6. Data from Phase I are included for comparison.

Figure 20: Uniaxial compressive strength versus porosity for tests conducted at -5°C (23°F).

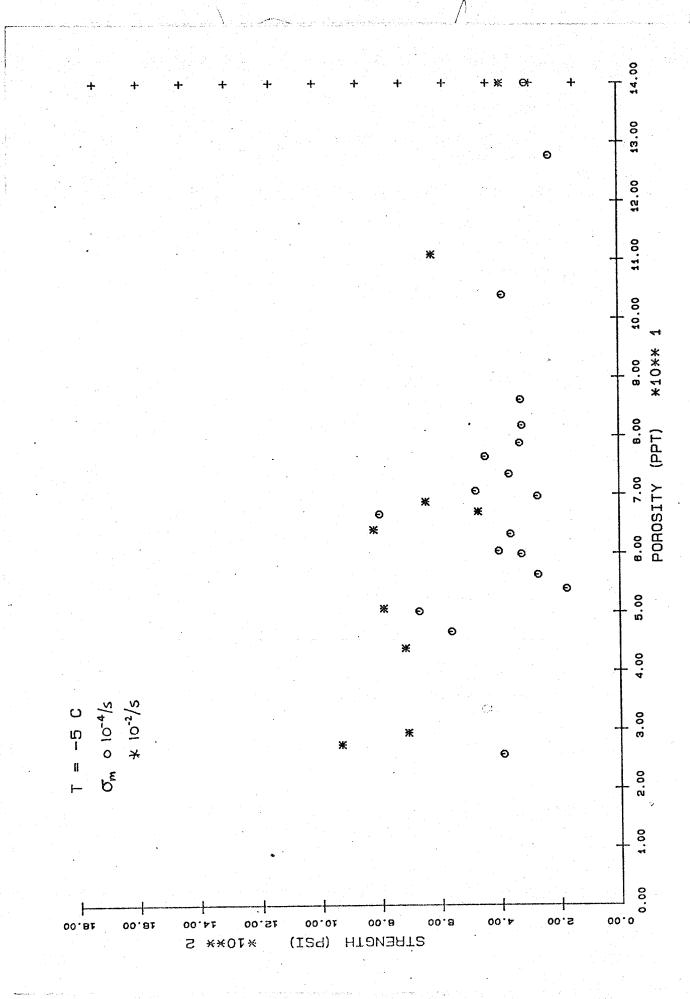


Figure 21: Uniaxial compressive strength versus porosity for tests conducted at -20°C (-4°F).

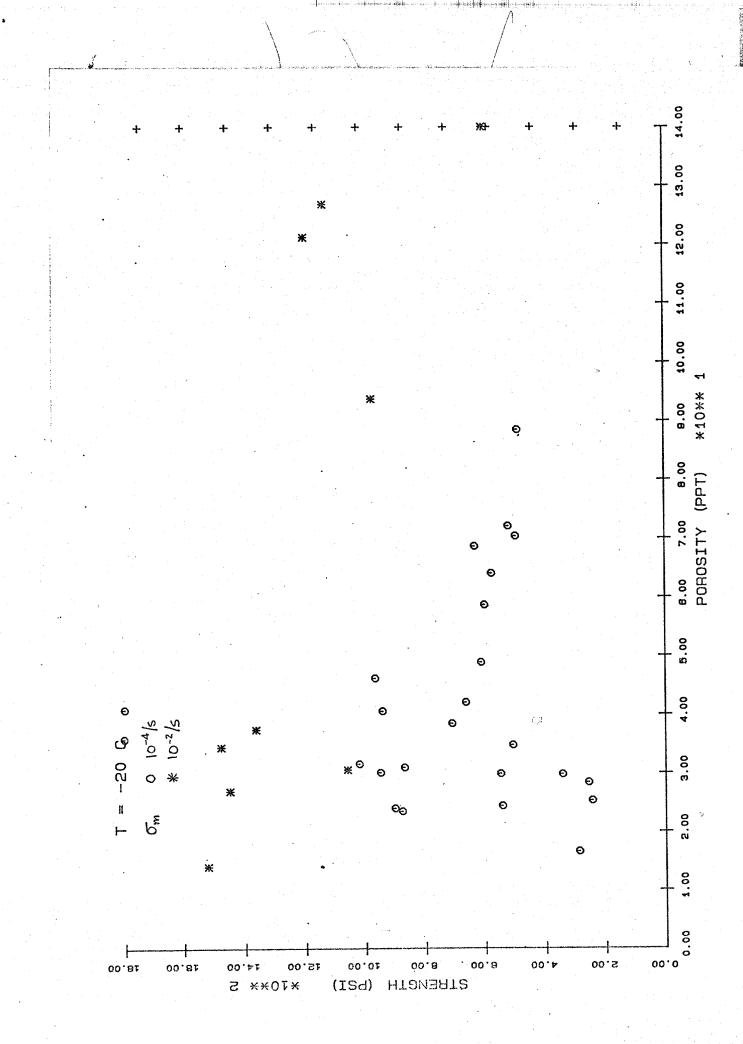


Figure 22: Uniaxial compressive strength versus porosity for horizontal and vertical samples tested at  $-5^{\circ}$ C (23°F) and  $10^{4}/s$ .

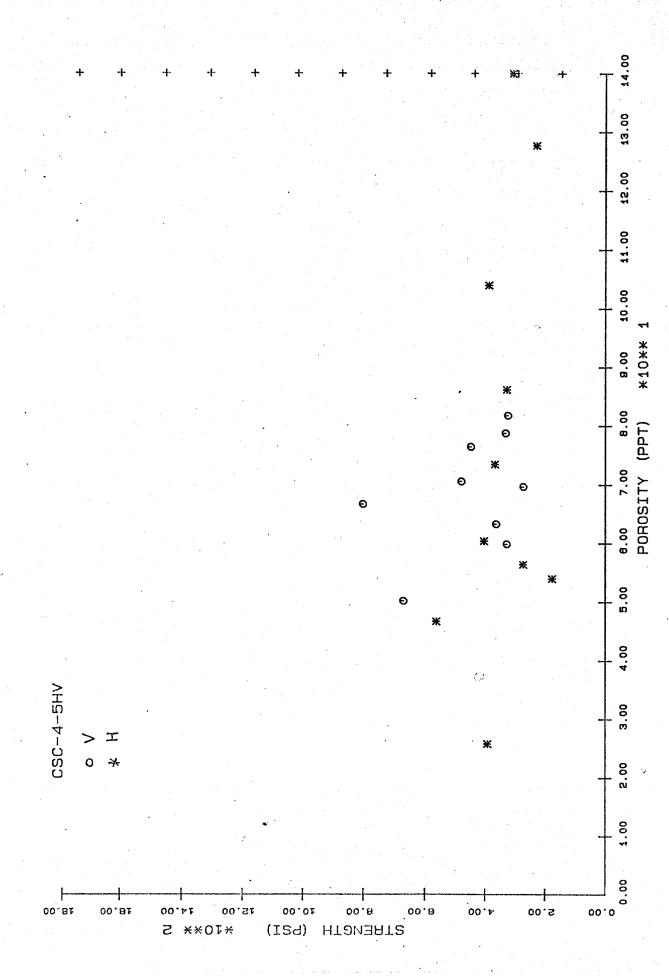


Figure 23: Uniaxial compressive strength versus porosity for horizontal and vertical samples tested at  $-20^{\circ}\text{C}$  (-4°F) and  $10^{4}/\text{s}$ .

{ ,l

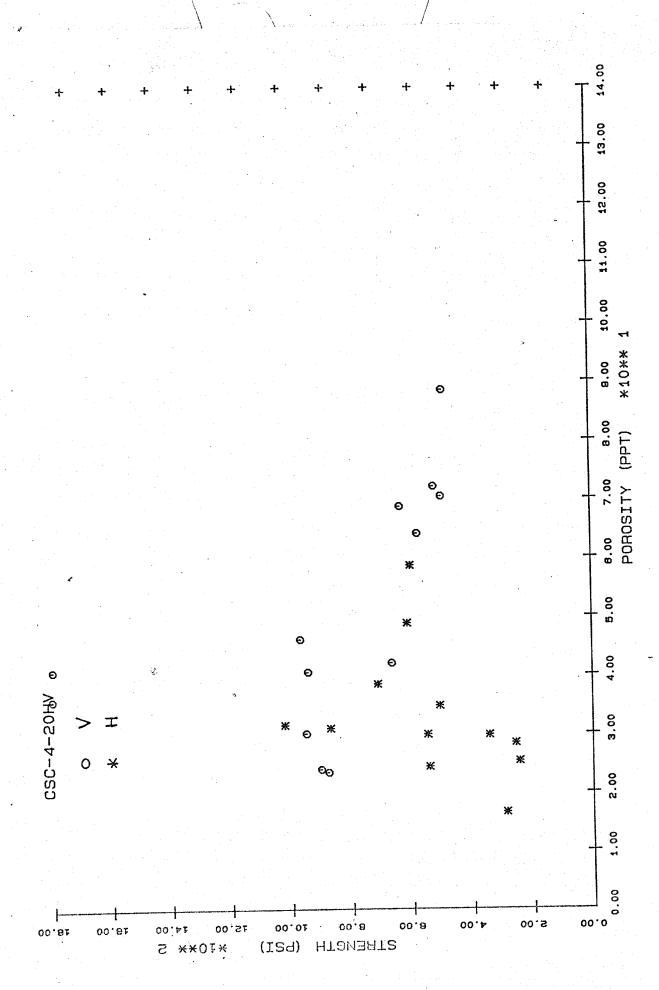


Table 6: Summary of residual/maximum compressive strength ratio data for Phases I and II.

# Residual/Maximum Strength Ratios

	Maximum	Minimum	Mean	Samples	Percent to 5% Strain
-5°C(23°F)					
$10^{-5}/s$ V	1.000	0.173	0.688±0.166	68	96
$10^{-4}/s V$	0.591	0.244	0.396+0.096	9	100
$10^{-4}/s$ H	0.794	0.245	0.439±0.159	10	100
$10^{-4}/s$ all	0.794	0.244	0.418±0.131	19	100
$10^{-3}/s$ V	0.421	.0.074	0.198±0.078	43	62
$10^{-2}/s \text{ V}$	-	<b>-</b> .	<u></u>	<del>-</del>	0
-20°C (-4°F)					
$10^{-5}/s V$	0.970	0.315	0.642±0.162	36	88
10 <sup>-4</sup> /s V	0.504	0.253	0.342±0.077	9	69
10 <sup>-4</sup> /s H	0.675	0.202	0.405±0.137	12	100
$10^{-4}/s$ all	0.675	0.202	0.378±0.114	21	84
$10^{-3}/s V$	0.746	0.047	0.194±0.148	18	44
$10^{-2}/s$ V	-	-	-		0

H - Horizontal

The results show that the residual strength/maximum strength ratio decreases with increasing strain-rate and is relatively insensitive to the ice temperature and porosity. As the strain-rate increases, fewer samples go to 5% strain and at  $10^{-2}/s$  all the tests terminated at the peak or maximum stress.

#### Failure Strain

Average sample failure strain at the peak or maximum stress for the different test conditions in Phases I and II are given in Table 7. The strains were calculated from the average of the DCDT measurements on the sample. In general, there is a strong tendency for the sample failure strain to decrease with increasing strain-rate. At low strain rates of  $10^{-5}$  and  $10^{-4}/s$ , the failure strain also decreases as the ice gets colder. However, at high strain-rates of  $10^{-3}$  and  $10^{-2}/s$ , the failure strain increases as the ice gets colder. Examination of the standard deviation of the mean strains indicate that the observed temperature trends are not statistically significant.

Strength versus strain to failure plots are given in Figures 24 and 25. At  $-5^{\circ}$ C there is a positive correlation between the strength and failure strain for the  $10^{-2}$ /s tests, whereas at  $10^{-4}$ s, there is no apparent correlation. At  $-20^{\circ}$ C, both the  $10^{-4}$ /s and  $10^{-2}$ /s tests show a positive correlation between the strength and failure strain.

Table 7: Summary of compressive failure strain data for Phases I and II.

## Failure Strain (%)

	Maximum	Minimum	Mean	Samples
-5°C (23°F)	·			
$10^{-5}/s$ V $10^{-4}/s$ V $10^{-4}/s$ H $10^{-4}/s$ all $10^{-3}/s$ V	0.83 0.62 0.26 0.62 0.20	0.06 0.09 0.06 0.06 0.05	0.38±0.17 0.18±0.17 0.12±0.07 0.14±0.12 0.13±0.03	71 9 10 19 69
10 <sup>-2</sup> /s V -20°C (-4°F)	0.10	0.02	0.07±0.02	9
$10^{-5}$ /s V $10^{-4}$ /s V $10^{-4}$ /s H $10^{-4}$ /s all $10^{-3}$ /s V $10^{-2}$ /s V	0.73 0.21 0.14 0.21 0.25 0.16	0.10 0.10 0.07 0.07 0.05 0.08	0.31±0.14 0.15±0.04 0.10±0.03 0.13±0.04 0.19±0.04 0.12±0.03	41 13 12 25 41 9

H - Horizontal

Figure 24: Uniaxial compressive strength versus failure strain for those tests conducted at -5°C (23°F).

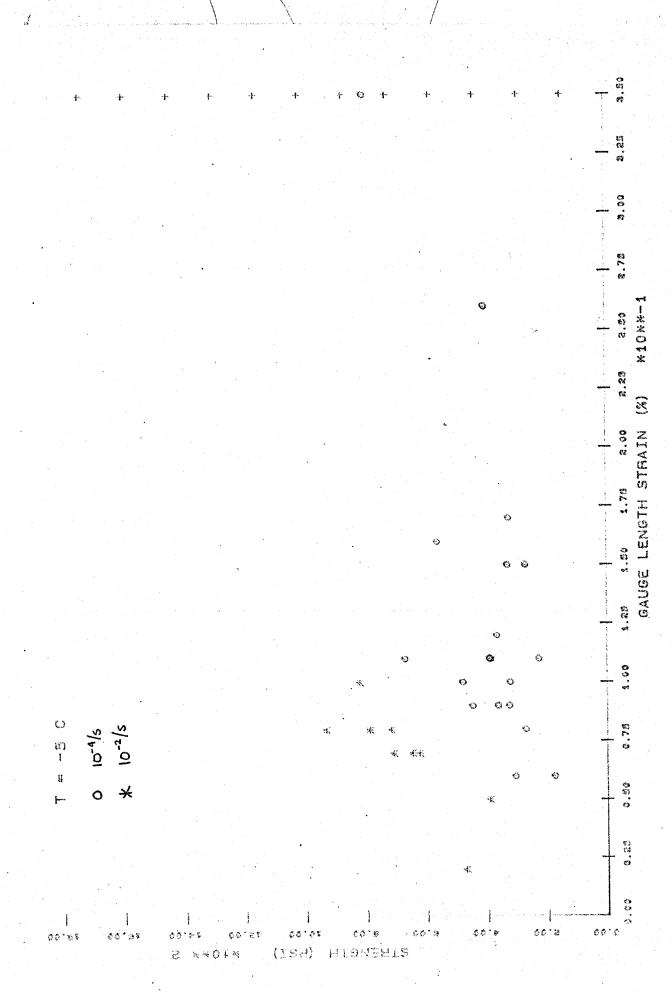
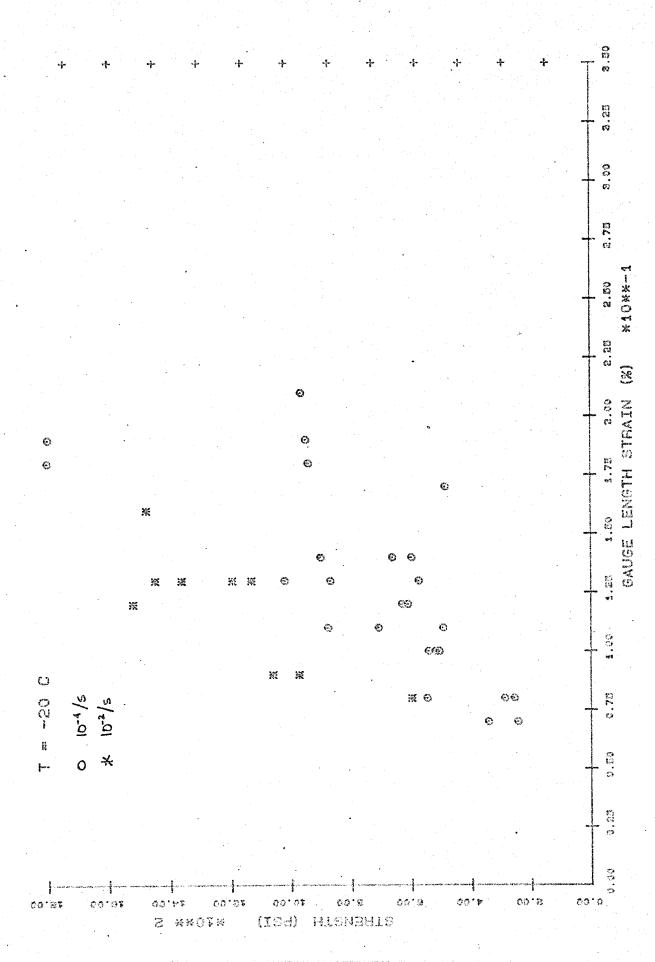


Figure 25: Uniaxial compressive strength versus failure strain for those tests conducted at -20°C (-4°F).



#### Initial Tangent Modulus

Estimates of the initial tangent modulus were obtain from the initial slope of the force-displacement curves using the same procedures as in Phase I. The results are plotted against strain-rate in Figures 26 and 27 and listed in Table 8. Modulus values from Phase I are included in both the figures and table for comparison. The initial tangent modulus is plotted against the ice porosity for ice temperatures of -5°C and -20°C in Figures 28 and 29, respectively.

It is interesting to note that the initial tangent modulus approaches the "dynamic" Young's modulus of the ice at a lower strain-rate in the colder -20°C tests. Furthermore, at a given strain-rate and temperature there is a tendency for the modulus to decrease with increasing porosity.

#### Poisson's Ratio

As in Phase I a MTS circumferential strain measurement kit was used to provide estimates of Poisson's ratio. At the conclusion of the compression tests the data were reduced and it was found that the Poisson's ratios from Phase II were much lower than those obtained in Phase I. For example, at  $-5^{\circ}$ C and  $10^{-4}$ /s the average Poisson's ratio at the start of the test was  $0.08 \pm 0.05$ . At  $-5^{\circ}$ C and  $10^{-3}$ /s in Phase I we obtained  $0.21 \pm 0.14$ . It was initially assumed that the Phase II values were lower because the samples were more porous and compressible. However, we were suspicious in that the Phase I samples having a high porosity did not generally have a low Poisson's ratio. Data reduction, calibration, and recording procedures for the circumferential strain measurements were first reviewed. As no

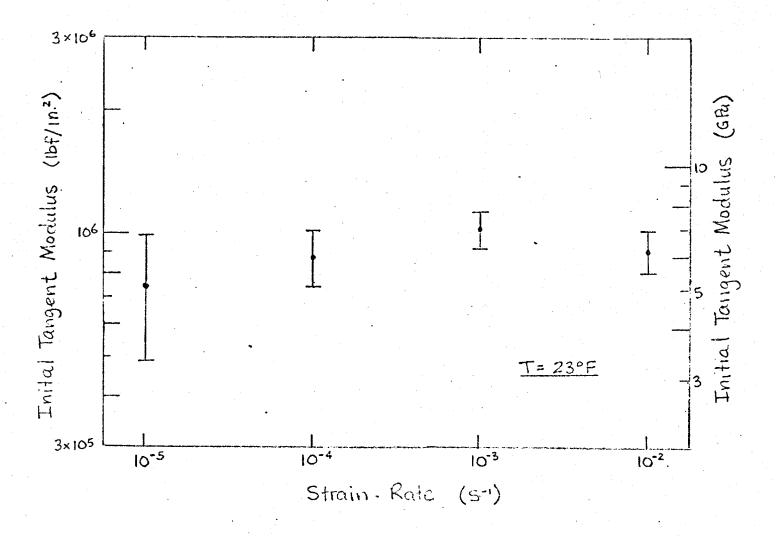


Figure 26: Initial tangent modulus in compression versus strain rate for those tests conducted at -5°C (23°F). The bars denote one standard deviation.

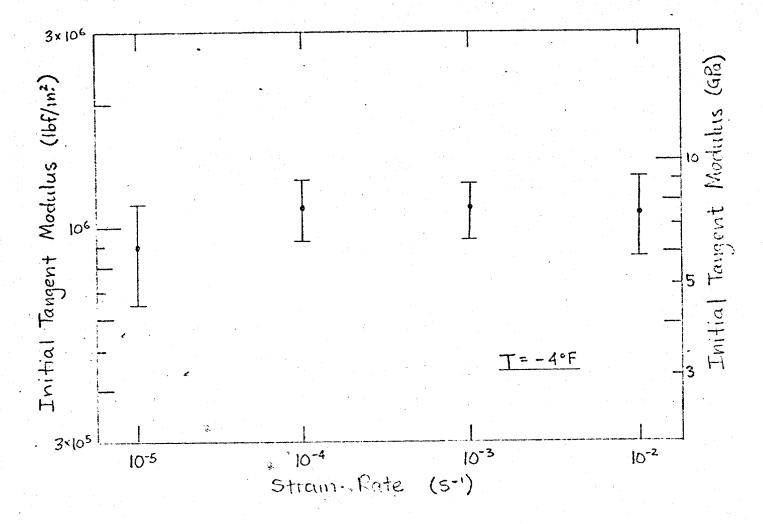


Figure 27: Initial tangent modulus in compression versus strain-rate for those tests conducted at -20°C (-4°F). The bars denote one standard deviation.

Table 8: Summary of compressive initial tangent modulus for Phases I and II.

### Initial Tangent Modulus

	Maximum		Minimum		Mean			* * *
	(	(1bf/ in. <sup>2</sup> x10 <sup>6</sup> )	(GPa):	(1bf/ in. <sup>2</sup> x10 <sup>6</sup> )	(GPa)	(1bf/ in. <sup>2</sup> x10 <sup>6</sup> )	Mean Porosity (ppt)	Samp1
	(Gra)	111. XIO /	(Gra)	THE XIV /	(010)		<u> </u>	
<u>-5°C (23°F)</u>								
$10^{-5}/s$ V	11.45	1.660	2.41	0.350	5.11±1.74	0.741±0.252	2 44	71
10 <sup>-4</sup> /s V	7.89	1.144	5.32	0.771	6.30±0.96	0.914±0.139		9
$10^{-4}/s$ H	7.41	1.074	4.41	0.639	5.81±0.94	0.842±0.136		10
$10^{-4}/s$ all	7.89	1.144	4.41	0.639	6.04±0.95	0.876±0.138		19
$10^{-3}/s \text{ V}$	14.00	2.030	4.95	0.718	7.07±1.38	1.025±0.200	) 46	71.
$10^{-2}/s$ V	6.90	1.000	4.89	0.709	6.21±0.73	0.901±0.106	68	9
-20°C (-4°F)	<u>)</u>					•		
10 <sup>-5</sup> /s V	13.79	2.000	3.45	0.500	6.14±1.68	0.890±0.244	36	41
10 <sup>-4</sup> /s V	9.70	1.406	5.35	0.776	7.74±1.42	1.122±0.206	5 50	13
10 <sup>-4</sup> /s H	10.28	1.490	6.18	0.896	7.58±1.26	1.099±0.182		12
$10^{-4}/s$ all	10.28	1.490	5.35	0.776	7.66±1.29	1.111±0.187		25
$10^{-3}/s$ V	10.38	1.570	4.89	0.709	7.62±1.19	1.105±0.173		40
$10^{-2}/s$ V	10.50	1.522	5.28	0.765	7.50±1.61	1.088±0.233	3 74	9
								T.

H - Horizontal

Figure 28: Initial tangent modulus in compression versus porosity for those tests conducted at -5°C (23°F).

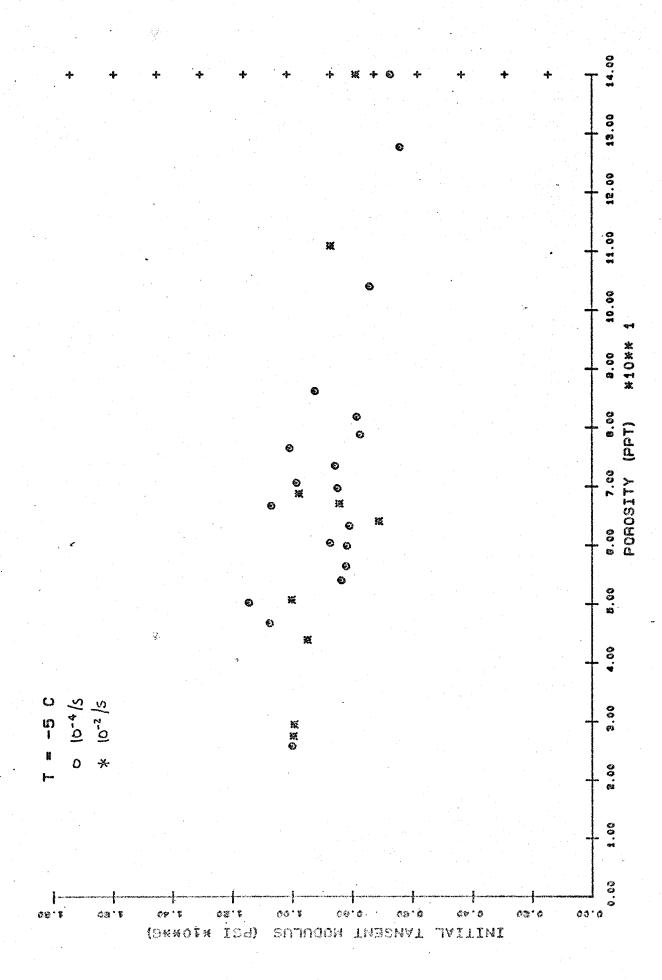
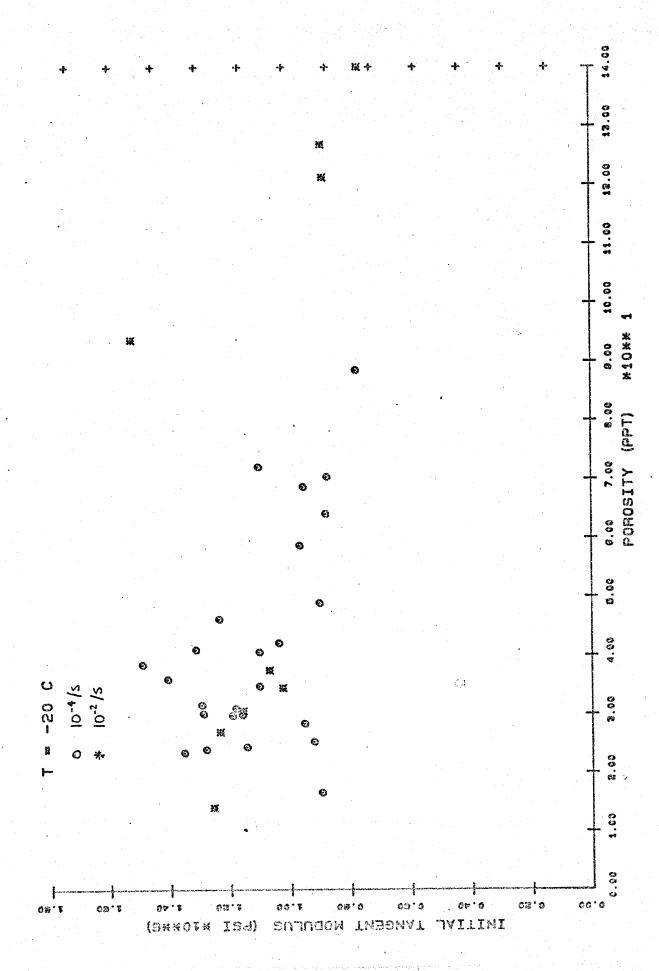


Figure 29: Initial tangent modulus in compression versus porosity for those tests conducted at -20°C (-4°F).



irregularities were found, eight Phase I specimens were tested. To our disappointment we again obtained low Poisson's ratio values. It appeared that we had a mechanical problem with the circumferential strain kit. An aluminum cylinder was then fabricated, instrumented with axial DCDTs and the circumferential strain kit, and tested on the MTS machine. While the measured Young's modulus was correct, Poisson's ratio was low and nonlinear. We found that to properly use the circumferential strain kit, it is necessary to support the circumferential strain extensometer. This did not prove to be an easy task and after numerous attempts with elastic bands, we stopped trying. Slight movements of the extensometer during loading resulted in erroneous measurements.

As a result of the above findings, no Poisson's ratio data will be provided in Phase II. Poisson's ratio data will also be removed from the Phase I report. In subsequent tests, it will be necessary to properly support the circumferential strain extensometer, or devise an alternate method to measure lateral deformation of the sample.

### UNIAXIAL TENSION TESTS

#### Sampling Scheme and Test Variables

Thirty-six constant strain-rate uniaxial tension tests were performed on vertically oriented multi-year pressure ridge samples in Phase II. The tests were conducted at two strain-rates,  $10^{-5}$ /s and  $10^{-3}$ /s, and at two temperatures, -20°C and -5°C. The number of tests at each test condition is summarized in Table 9. Details on the sample preparation and testing

Table 9: Number of uniaxial tension tests at different temperatures and strain-rates.

10 <sup>-5</sup> /s	10 <sup>-3</sup> /s		
9V	9V	18V	
9V	9V	187	
18V	18V	36V	
	9V 9V	9v 9v 9v 9v	

techniques are given in Mellor et al. (1983). The procedures used in Phase II were identical to those in Phase I with the exception that, for on the ice axial strains, the DCDT gauge length was increased from 4.0 in. (10.2 cm) to 4.5 in. (1.4 cm). In Phase I tension tests were performed on samples from a multi-year floe and this data cannot be grouped with the Phase II ridge data.

### Uniaxial Tensile Strength

A detailed tabulation of the results from the constant strain-rate uniaxial tension tests is given in Appendix B. The average tensile strength of the ice is plotted against strain-rate in Figures 30 and 31. Figure 30 contains the results from those tests conducted at -5°C (23°F), and Figure 31 presents the results from those tests conducted at -20°C (-4°F). The bars denote one standard deviation from the mean. Average tensile strength values are also listed in Table 10.

In general, the tensile strength shows very little variation with strain-rate or temperature. The lower mean strength obtained at  $10^{-3}$ /s and  $-5^{\circ}$ C (23°F) is probably due to the higher porosity of the samples.

The tensile strength is plotted against the ice porosity in Figures 32 and 33. Disregarding variations in the ice structure there is a tendency for the ice strength to decrease with increasing porosity.

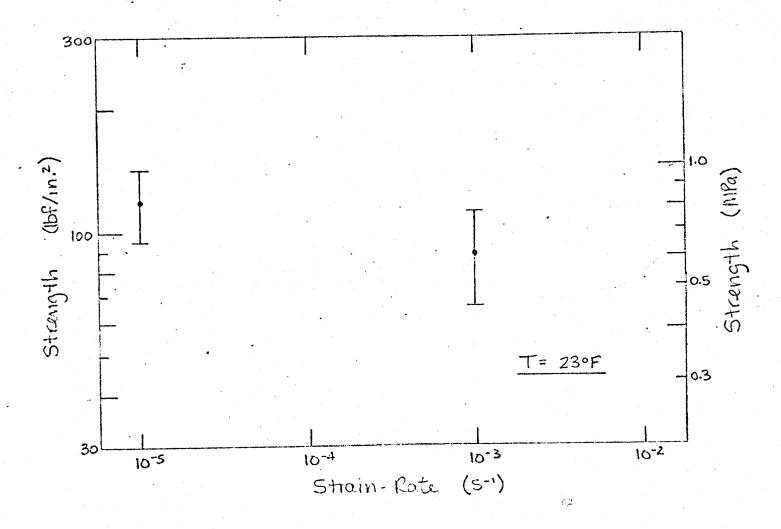


Figure 30: Uniaxial tensile strength versus strain-rate for those tests conducted at -5°C (23°F). The bars denote one standard deviation.

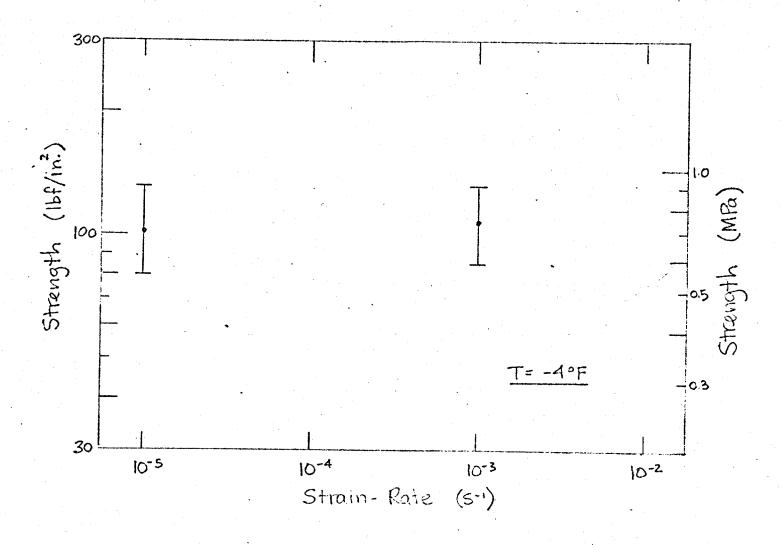


Figure 31: Uniaxial tensile strength versus strain-rate for those tests conducted at  $-20\,^{\circ}\text{C}$  (-4°F). The bars denote one standard deviation.

Table 10: Summary of tensile strength data for Phases II.

## Uniaxial Tensile Strength

	Maxit		Minim (MPa) (1b	um f/in. <sup>2</sup> )	(MPa) Mea	in (1bf/in. <sup>2</sup> )	Mean Porosity (ppt)	Samples
-5°C (23°F)	<u>)</u>		• • • • • • • • • • • • • • • • • • • •					
$10^{-5}/s$ V $10^{-3}/s$ V	1.03 0.83	149 120	0.57 0.41	82 60	0.82±0.17 0.61±0.16	119±24 89±23	78 108	9 9
-20°C (-4°I	<u>?)</u>					•		
$10^{-5}/s$ V $10^{-3}/s$ V	0.92 0.92	134 134	0.49 0.48	71 69	0.71±0.16 0.75±0.16	103±23 109±23	82 77	9 9

V - Vertical

Figure 32: Uniaxial tensile strength versus ice porosity for those tests conducted at -5°C (23°F).

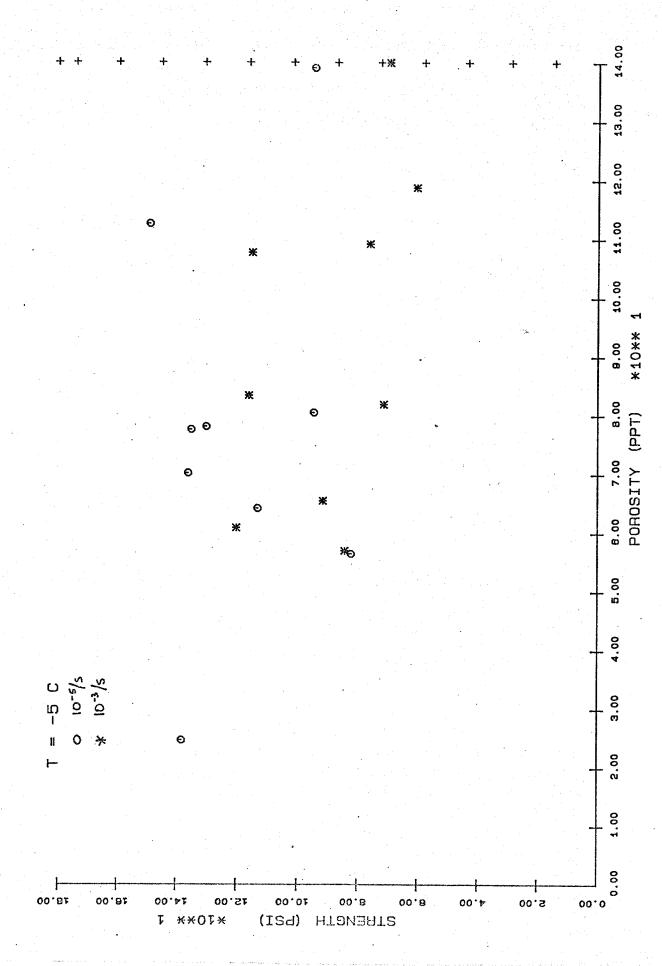
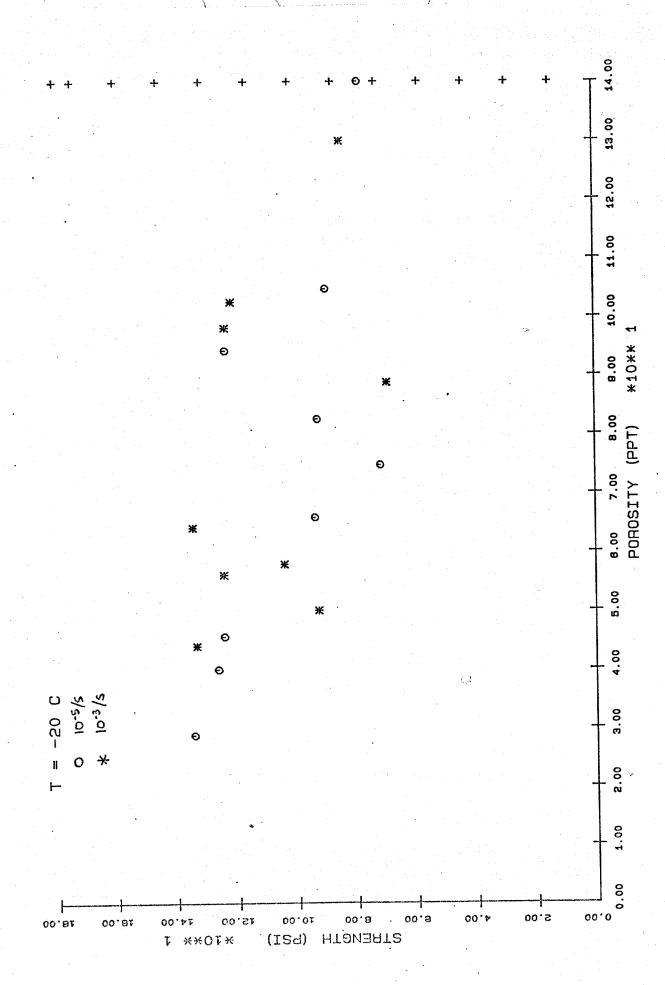


Figure 33: Uniaxial tensile strength versus ice porosity for those tests conducted at -20°C (-4°F).



#### Failure Strains

Average tensile failure strains at the peak or maximum stress for each test condition are given in Table 11. In general, the samples failed at 0.01 to 0.02% strain.

### Initial Tangent Modulus

Estimates of the initial tangent modulus were obtained from the initial slope of the force-displacement curves. The results are plotted against strain-rate in Figures 34 and 35 and listed in Table 12. The modulus is also plotted against the ice porosity in Figures 36 and 37.

The initial tangent modulus shows a slight increse with increasing strainrate, and a slight decrease with increasing temperature and porosity. Relative to the compressive initial tangent modulus variations are small.

Table 11: Summary of tensile failure strain data for Phase II.

Failure	Strain	(%)

	Maximum	Minimum	Mean	<u>Samples</u>
-5°C (23°F)				
$10^{-5}/s$ V $10^{-3}/s$ V	0.022 0.013	0.014 0.007	0.019±0.002 0.010±0.002	9 9
-20°C (-4°F)	<del>-</del> .			
$10^{-5}/s$ V $10^{-3}/s$ V	0.022 0.012	0.009 0.009	0.013±0.004 0.011±0.001	9 9

V - Vertical

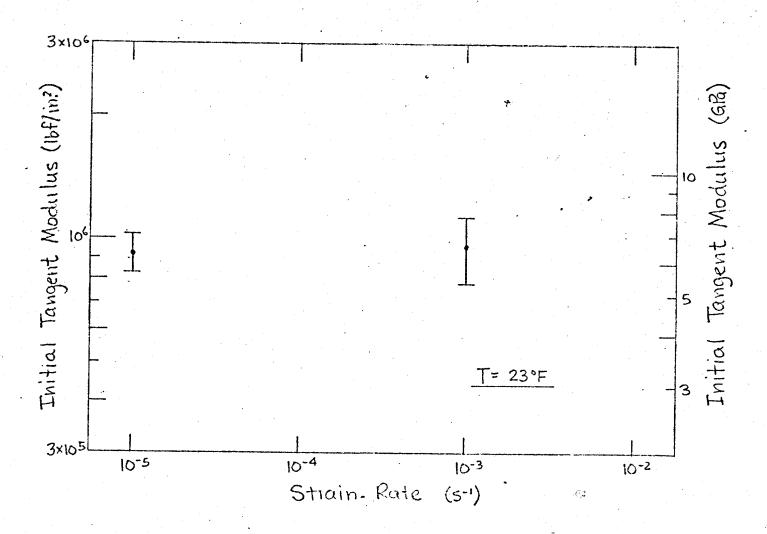


Figure 34: Initial tangent modulus in tension versus strain rate for those tests conducted at -5°C (23°F).

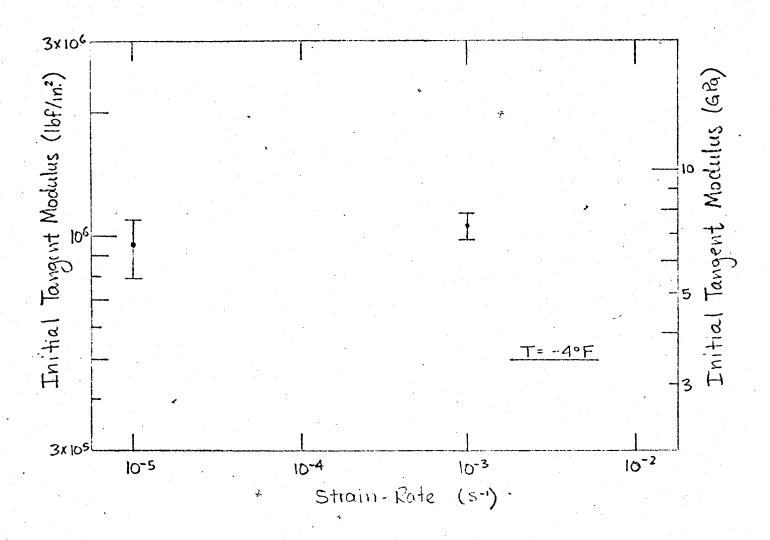


Figure 35: Initial tangent modulus in tension versus strain-rate for those tests conducted at -20°C (-4°F).

Table 12: Summary of tensile initial tangent modulus data for Phase II.

## Initial Tangent Modulus

	(	imum 1bf/ n. <sup>2</sup> x10 <sup>6</sup> )		11mum (1bf/ in. <sup>2</sup> x10 <sup>6</sup> )	(GPa)	$\frac{100}{(1000)}$ 10. $\frac{1000}{(1000)}$	Mean Porosity (ppt)	<u>Samples</u>
-5°C (23°F)					•			
$10^{-5}/s V$ $10^{-3}/s V$	7.59 8.32	1.100 1.207	5.42 4.25	0.786 0.616	6.39±0.68 6.60±1.19	0.927±0.099 0.957±0.173	78 108	9
-20°C (-4°F)								
$10^{-5}/s$ V $10^{-3}/s$ V	7.82 8.12	1.134 1.177	4.17 6.59	0.604 0.955	6.54±1.12 7.31±0.54	0.949±0.162 1.060±0.079		9 9

V - Vertical

Figure 36: Initial tangent modulus in tension versus porosity for those tests conducted at -5°C (23°F).

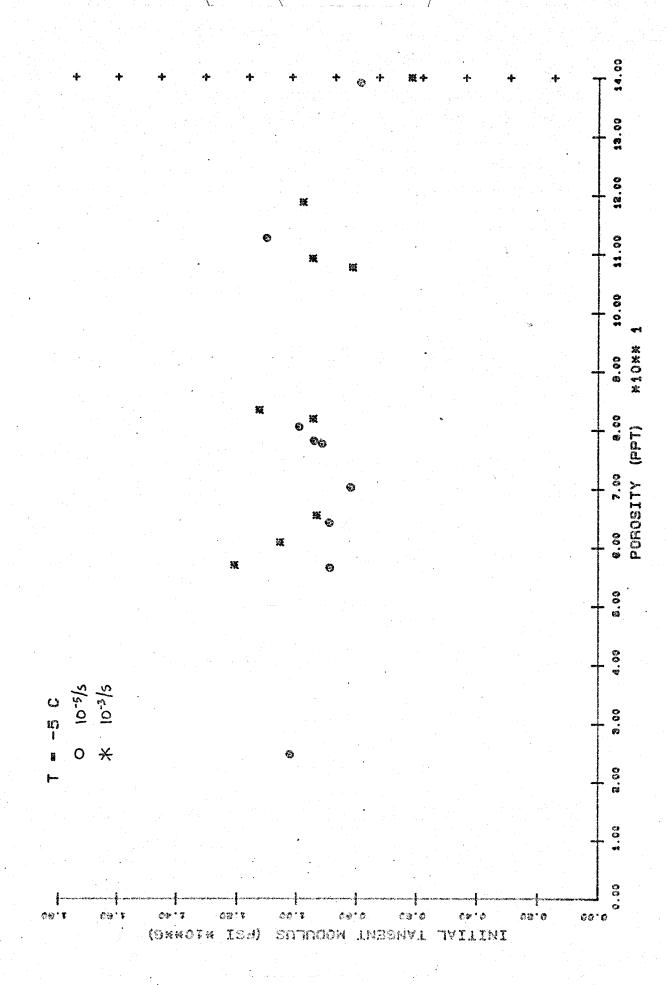
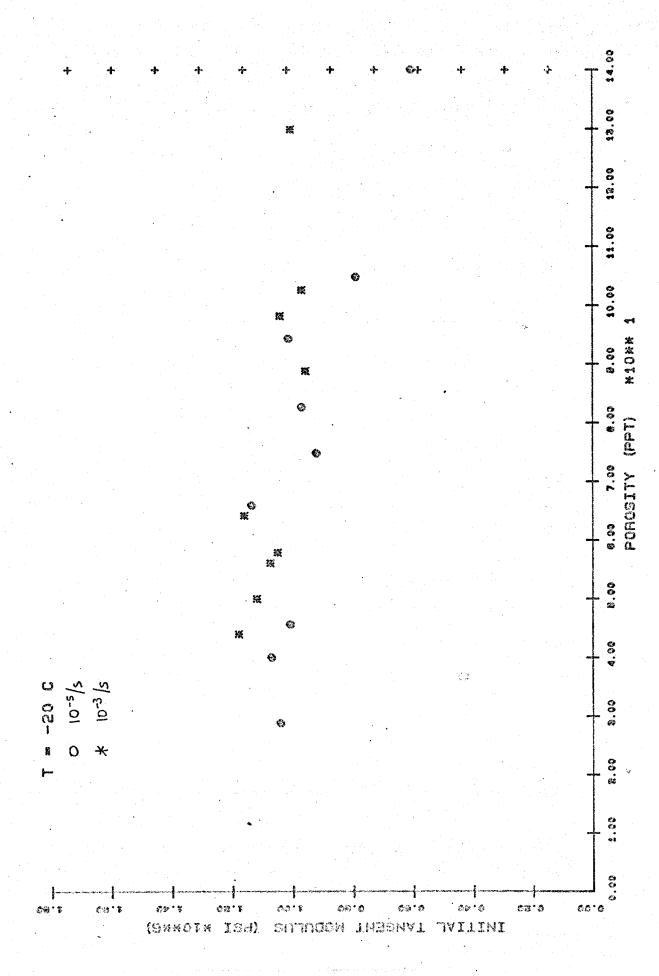


Figure 37: Initial tangent modulus in tension versus porosity for those tests conducted at -20°C (-4°F).



### APPENDIX A

# Constant Strain-Rate Compression Data

This appendix contains the results from the constant strain-rate, uniaxial compression tests (CSC). The parameters listed for each test are defined in the Index. CSC - 4-5 denotes those tests conducted at a strain-rate of  $10^{-4}/\mathrm{s}$  and a temperature of  $-5^{\circ}\mathrm{C}$  (23°F), etc.

Column No.	Symbol	Description
1 2	σ <sub>m</sub> , psi ε <sub>m</sub> (GL),%	Peak Stress Strain at $\sigma_m$ determined by the DCDT's over a guage length of 5.5 inches
3	ε <sub>m</sub> (FS),%	Strain at $\sigma_m$ determined by the extensometer over the full sample length of 10 inches
4	t <sub>m</sub> , sec	Time to peak stress
5	σ, psi	Stress at end of test
6	ε (FS),%	Full sample strain at end of test
7	t <sub>e</sub> , sec	Time to end of test
8	E <sub>i</sub> (GL),x10 <sup>6</sup> psi	Initial tangent modulus determined using strains found over the gauge length
9	E <sub>O</sub> (GL),x10 <sup>6</sup> psi	Secant modulus determined using gauge length strains
10	E <sub>o</sub> (FS),x10 <sup>6</sup> psi	Secant modulus determined using full sample strains
11	s,°/00	Sample salinity at test temperature
12	o, 1b/ft <sup>3</sup>	Sample density at test temperature
13	V <sub>b</sub> ,°/°°	Brine volume at test temperature
14	V <sub>a</sub> ,°/ <sub>00</sub>	Air volume at test temperature
15	n,°/00	Porosity at test temperature
16	σ <sub>e</sub> /σ <sub>m</sub>	Ratio of end to peak stress
17	lce squarenéss, in	Sample squareness after ends are milled
18	End cap squareness, in	Sample squareness after endcaps are mounted
19	Shim, in	Amount of shim stock inserted between low end of sample and actuator before testing
20	V <sub>o</sub>	Initial Poisson's Ratio; circumferential and gauge length strain measurements used
21	vm	Poisson's ratio at peak stress; $\epsilon_{m}(C)$ and $\epsilon_{m}(GL)$ used
22	$\varepsilon_{\mathrm{m}}^{\mathrm{m}}(\mathrm{C}),\%$	Strain at $\sigma_m$ determined by the circumferential extensometer

Also Note: FILE B-3-5 indicates Below Level Ice  $10^{-3}/\text{sec Strain Rate}$   $-5^{\circ}\text{C Test Temperature}$ 

780 0.650 0.34 48.69 0.3 150.2 150.6 0.010 0.006 0.006 0.02 0.07 0.04	12.0 98.9 110.9	011 2,360 0.85 56.15 8.2 21.3 29.5 0.004 0.010 0.014 0.08 0.14 0.01	0.08 0.11 0.947 0.895, 0.895 1.44 55.70 13.8 30.0 43.9 0.010 0.002 0.002 0.04 0.11 0.01	•		•	RC231-278/305 931 0.080 0.100 0.12 931 0.10 0.12 0.995 1.164 0.931 1.84 56.92 18.1 9.5 27.5 0.004 0.003 0.005 0.06 0.15 0.01
•							
•							
•							
•							
•							
0.04 48.69 0.3 150.2 150.6	1.35 51.74 12.0 98.9 110.9	0.85 56.15 8.2 21.3 29.5	44 55.70 13.8 30.0 43.9	55.00 23.3 43.8 67.1	54.64 3.7 47.0 50.7	5.00 24.8 44.0 68.8	92 1841 9.5 27.5
0.04 48.69 0.3 150.2	1.35 51.74 12.0 98.9	0.85 56.15 8.2 21.3	44 55.70 13.8 30.0	55.00 23.3 43.8	54.64 3.7 47.0	5.00 24.8 44.0	92 1841 9.5
0.04 48.69 0.3	1.35 51.74 12.0	0.85 56.15 8.2	44 55.70 13.8	55.00 23.3	54.64 3.7	5.00 24.8	92 1841
n.84 48.69	1.35 51.74	0.85 56.15	44 55.70	55.00	54.64	5.00	92
, 40.0 0	1.35	0.85	4		44.7	S	56
			**	2.46	0.39	2.61	1.84
3.650	3.565	2.368	3.895	3.783	0.716	0.717	0.931
.780	1.887	.011	895,	. 353	.985	.921	1.164
390 0.06 0.07 0.785 0.	1.869	708 0.03 0.09 0.992.1.	0.947	3.840 2	000.1	975 (	0.995
0.07	0.11	0.09	0.11	0.08	0.13	0.18	0.12
90.0	0.11	£0.0 €	0.08	90.0	0.11	60.0	0.10
390	621	708	716	470	788	645	931
0.07	0.11	0.09	0.11	0 • 0 8 ·	0.13	0.12	0.12
	110	.030	•080	090•	.110	060•1	1.100
RA206-131/158	•		<i>(</i> )	$\circ$	ر. ص	24	305 80 0
	090	.060	1.060	0.060 0.110 0.030	0.060 0.110 0.030 0.080 0.080	58 0.060 54 0.110 57 0.030 60 0.080 69 0.060	

FILE CSC-2-5

•	18 19 20 21 22	0.6 0.006 0.08 0.34 9.05	10 0.010 0.05 0.10 0.01	05 0.005 0.04 0.31 0.0	12 0.012 0.02 0.77 0.28	03 0.003 0.09 0.08 0.01	16 0.016 5.06 0.16 0.01	00 0.004 0.19 0.40 0.04	05 0.005 0.13 0.13 0.08	09 0.009 0.05 0.12 0.01	03 0.003 0.13 0.33 0.0	04 0.004 0.14 0.23 0.01	09 0.006 0.05 0.05 0.01			15 0.013 0.03 0.09 0	15 0.013 0.03 0.09 0 11 0.011 0.06 0.11 0 41 0.041	5 0.013 0.03 0.09 0 1 0.011 0.06 0.11 0 1 0.041 3 0.013 0.07 0.23 0	5 0.013 0.03 0.09 0 1 0.011 0.06 0.11 0 1 0.041 5 0.013 0.07 0.23 0 1 0.001 0.09 0.34 0
	15 16 17	78.8 0.591 4.003 0.0	7 104.6 0.578 0.048 0.01	70.6 0.433 0.007 0.0	1 60.4 0.505 0.090 0.01	63.3 0.362 0.009 0.3	5 86.2 0.377 0.027 0.01	9 127*7 0*403 0*033 0*00	8 66.7 0.309 0.036 0.60	25.8 0.245 0.035 0.C	2 69*7 0*424 0*006 0*30	54.0 0.794 0.013 0.0		/6.5 V.428 V.VV8 V.V	76.5 U.428 U.008 U.0 56.4 0.410 0.033 C.0	56.4 0.410 0.033 C.0 81.8 0.394 0.005 5.0	6.5 0.428 0.0033 0.05 56.4 0.410 0.033 0.0 81.8 0.394 0.005 0.0 142.2 0.363 0.045 0.0	6.5 0.428 0.003 0.05 56.4 0.410 0.033 0.0 81.8 0.394 0.005 0.0 142.2 0.363 0.045 0.0 50.2 0.244 0.007 0.0	56.4 0.410 0.033 C.0 81.8 0.394 0.005 0.0 142.2 0.363 0.045 0.0 50.2 0.244 0.007 0.0
	13 14	10.8 68.0	4.3 99.7	29.0 41.6	17.3 43.1	8.6 54.6	11.7 74.5	10.8 116.9	24.9 41.8	18.2 7.6	28.5 41.2	25.5 28.5	ç	r	.2 31	.2 31	.2 31 .6 72 .6 139	. 2 31 . 6 72 . 5 139	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
	11 12	1.17 53.50	6.48 51.62	3.75 55.18	1.82 54.98	0.92 54.24	1.28 53.13	1,23 50,69	2.62 55.12	1.85 57.63	2,99 55,19	2.54 55.89	200	24.43 67.63	•62 55•74	.62 55.74 .65 55.25	62 55.74 65 53.25 79 49.32	62 55 74 65 65 74 65 65 65 74 65 65 65 65 65 65 65 65 65 65 65 65 65	62 55.74 65 53.25 70 49.32 83 56.25 91 57.19
	80 80 10	71 6.226 6.194	39 6.351 0.351	83 6.478 0.531	71 (-155 0-149.	07 0.462 0.453	21 0.362 0.815.	39 0.206 0.206	68 0.129 0.200	99 0.355 0.433	46 6.181 0.226	34 0.292 0.194	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	10 C C = 10 C C C C C C C C C C C C C C C C C C	18 0.339 0.3	18 0.339 0.3 82 0.322 0.3	18 0.339 0.3 82 0.322 0.3 71 0.510 0.3	18 0.339 0.3 82 0.322 0.3 71 0.510 6.3	18 0.339 0.3 82 0.322 0.3 71 0.510 0.3 44 0.608 0.9
0	. (7) .	2.0 5.068 93.67	2.0 3.008 98.8	5.00 530.3 0.9	5.63 569.3 6.8	୫•0 ଅ•00ୟ ଓଡ•ର	5*00 0*00 00*3	9*0 0*00\$ 0 <b>0</b> *s	5.00 500.9 1.6	5.06 580.2 0.8	5.00 500.0 0.8	୫•୦ ୧•୦୦୫ ୧୦•୫	0.0 550.0		• 0 5 0 0 0 0 0 0 •	0 2 2 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			
2	 R	155	223	207	203	131	123	c.	247	95	115	139	161		111	111	111	111 127 111	111 127 111 163 151
	03 04	70 16.55	10 10.60	99 7.89	70 25.40	89 7.10	40 5.90	10 11.70	00 37.70	90 5.80	20 14.00	90 5.50	10 9.30		180 6.60	80 6.6 90 9.0	80 6.6 90 9.0 80 6.3	80 6.6 90 9.0 80 6.3	80 6.6 90 9.0 80 6.3 70 6.0
	SAMPLE # 02	R32-133/160V 333 0.150 0.1	843-150H 386 0.110 0.13	833-205/232V 478 0.100 0.00	R43-222H 462 3-263 3-2	846-647/073V 362 0.038	R44-973H 326 0-090 0-04	R44-060H 227 0.110 0.13	846-083/110V 800 0.629 0.48	R44-086H 390.0.110 0.0	846-147/173V 271 0.150 0.13	844-156H 175 0.060 0.0	846-246/272V 446 0.090 0.1		. R44-256H 271 0-380 0-0	44-256H 271 0.380 0.0 47-625/053V 322 0.100 0.0	44-256H 271 0.380 0.0 47-25/053V 43-22 0.100 0.0 45-240H	44-256H 271 0.380 0.0 47-125/053V 322 0.100 0.0 45-740H 336 0.360 0.0 47-151/217V	44-256H 47-1257053V 45-220.1000.0 45-60H 47-1517217V 6690.1100.0

163 5.00 500.0 0.854 0.305 0.282 2.16 54.43 20.3 53.3 73.5 0.445 0.034 0.015 0.015 0.09 0.45 0.05

844-298H 366 0-120 0-130 15-00

	L.E. CS	21111	J)												
SAMPLE # 02 63	6.0	ស្ត	26 97	68 69	10	end end end	2 13	14	15 16	17	18 19	20	12.	22	
R32-231/258V 0.283 22.	53 4	, 485 G	3.83 558.8 1.2	33 7.459	0.344	3.76 55.6	68 12.2	34.0	46.2 0.504	0.000	000-0 000	0.21	0.50	0.10	÷
843-245H 545 0.080 0.100 9.	50 1	167 5	\$.00 \$80.0 I.1	59 0.683	0.546	3.57 56.4	6 10.1	19.8	29.9 0.306	0.042 0.	012 0.009	0.25	0.48	0.04	
R32-267/294V . 661 0.146 3.220 21.		167 5	5.00 509.0 1.0	36 0.472	0.301	3.76 55.9	2 12.3	52.9	42.1 0.253	0.012 0.	010 0.010				
R33-268/295V 899 0.140 0.170 13.	50	334 1	1.40 13.50 1.2	278 8.642	0.529	4.35 57.1	3 14.5	4.6	24.0	0.015 0.	001 0.001	0.16	8 2 3	50.0	
843-285H 738 0.110 0.100 9.	.70 1	143 5	5.05 500.0 1.4	490 0-644	0.738	3.39 56.0	4 11.1	27.4	38.5 0.202	ð.833 C.	900*0 900	60.0	0.44	0 • 0 5	
R32-303/528V 573 0.130 0.110 11.	7.9	223 5	•0 2•005 30•	878 0.441	0.521	1.48 54.0	1.4 6	59.4	64.1 0.389	0.010 0.	300°0 500	0.15	6.59	80•0	
843-316H 342 0.070 0.090 8.	© (2)	231 5	5-90 500-0 1-1	191 0.489	0.380	3.71 56.6	3 12.3	17.5	29.8 0.675	0.025 0.	C27 0.027	60.0	5.32	20.0	
R32-343/369V 6-240 23.	ري دي	175 5	ର ଜଣ ଜଣ ଜଣ ଜଣ ଅନ୍ତର	776 0.285	0.202	2,51,52,5	92 7.8	80.7	88.5 0.361	0.000.0	015 0.015	0.21	0.47	0.08	
R43-357H 597 0.140 0.130 14.	0 4	239 6	9*0 0 2000 ° 0 0*9	965 0.426	0.459	1.67 54.4	15 5.3	53.4	58.7 0.400	0.038 0.	9000 900	0.19	0.42	90 • 0	
833-242/268V 947 0.190 0.190 19.	40	700 0	0.59 61.00 1.2	287 0.498	0.498	5.91 56.	94 16.7	13.4	30.1	0.005 6.	៩០០•០ ទី០០	0.17	0.31	90 • 6	
843-257H 541 0.100 8.100 11.	ى ق 2	215	5.60 500.0 1.	144 0.541	0.541	3.61 56.	92 12.0	12.4	24.4 0.397		0.008	0.18	0.52	3 • 0	
R33-368/395V 7 939 0.180 0.160 14.	.20	247	5.00 599.0 1.1	101 0.522	C.587	4.52 56.	20 14.8	25.7	40.6 0.263	0.000.0	004 0.004	0.17	0.70	5.12	
R43-341H 867 0.138 0.158 14.	0 K.	207	5.68 508.3 1.	179 6.667	0.578	0.44 55.	74 1.3	29.7	31.0 0.239	0.025 0.	003 0-003	0.26	6.59	ଅଧ୍ୟ ପ	
46-121/147V -160 10 517 0-160 0-160 10	50	163	5.35 500.5 1.	101 0.517	6.517	2.58 53.	89 8 1	64.0	72.1 0.315	0.012 0.	930-6 905	0.14	0.43	43.5	
844-128H 255 0.080 0.090 9.		151	୫-୧୧ ୭୬ଅ-୧ ପ•	952 0.319	0.283	3.27 56.	60, 10.8	17.6	28.4 0.552	0.037 0.	900 0 600°	0.23	0.37	0.03	
846-173/199V 493 0-110 0-120 12-	3	175	୍ଧ ଓ କଳ୍ପର ଜଣ ବଳ	373 0.448	0.411	1.70 53.	77 5.3	65.1	70.4 0.355	0 600°0	.008 0.008	0.23	0 • 4 C	0.0.4	
R44-186H 1018 0-136 0-110 12	649	64 67 64 64	5.68 535.3 1·	292 0.783	0.926	3.69 56.	51 12.1	19.4	31.6 0.297	0.024 0.	900*0 900	0.16	0.23	0 • 03	
346-276/333V 629 0.129 0.120 13	2	223	2.00 500 500.0	952 0.524	0.524	1.02 53.	72 3.2	65,5	68.7 0.355	0.006.0	.005 0.005	0.21	0.59	8.05	
844-259H 659 0.120:0.120 13	13 13	589	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	សម្រុក សមាន ខេត្ត សមាន ខេត្ត	ස ග ග	1.00 54.	99 5.1	43.8	48.9 0.442	0.058 0.	013 0.013	0.17	0.38	0.05	
847-090/116V 1798 6-190 6-210 20	.73 1	297	5.27 25.73 1.	311 0.946	3.856	3.49 55.	92 11.4	29.6	41.0 %	0.026 9.	.002 0.002	0.19	0.42	80.0	
R44-103H 505 0.100 0.580 7	k.) C.)	के 51	• (1) • (1) • (2) • (3) • (3) • (4) • (5)	3 C C C	5 6.631	3.00	18 10.1	24.7	34.8 0.386	0 4 0 2 5 0 °C	300°0 900°				
844-116H 243 6-070 3-070 6	9•	T FF	**************************************	921 9.347	79.347	3, 71, 56.	79 11.3	14.3	25.3 0.457	0.010	• 002 0°• 002•	0.21	0.47	0.04	
847-127/153V 1846 0-180 0-125 14	.60 1	9 <b>4</b> 3	0.12 14.60 1.	456 1.226	828°T	3.28 55.	16 10.8	25.2	36.0	0.011.0	.004 0.004	9.14	0.34	0.06.	
844-141H 287 0.086 0.116	( · '₩\$	(3) 4 ) e-4		532°0 96×	0.261	3.54 57.	35 11.	p 4.7	16.6 0.470	0 -040 0	.003 0.003	0.23	0.32	0 0 0 5	
847-302/3259V 875 0.110 0.130 10	ි ල •	247	• E	352 0.79	6 0.875	2.87 56.	.71 8.	5 15.0	23.5 0.282	0.0005	600-0-600-	0.18	0 8 0	80 <b>.</b>	

### APPENDIX B

### Constant Strain-Rate Compression Data

This appendix contains the results from the constant strainrate, uniaxial tension tests (CST). The parameters listed for each tests are defined in the Index.

 $\circ$ 

;	20			-	•	. :				
	<u>σ</u> ,			•• • •		•				
		• 002	.001	*005	.010	-007	.003	.003	100.0	800°0
	11	0.006 0.005	0.005 0.001	0.007 0.002	0.003 0.010	0.011 0.007	0.003 0.003	0,005 0,003	0.015 0.007	0.005 0.008
	16									
	13 14 15 16	65.8 65.9	94.3	40 <b>4</b> 0	200.1	82.6	0.0 104.8 104.8	1.5 73.3 74.8	10.4 18.4 28.8	42.5 45.6
	14		1.6 92.7 94.3	4.6 35.4	0.0 200.1 200.1	0.2 82.4	104.8	73.3	18.4	42.5
	13	0.1	1.6	4.6	0.0	0.2	0.0	1.5	10.4	3.1
	12	.03 53.54	.51 52.12	41 55.46	.01 45.93	.98 52.69	0.81 51.40	.47 53.23	3.16 56.84	1.97 55.03
	11 01 66 80 . 10	1.134 0.935 0.935 0.03 53.64	1.011 0.879 0.879 0.51 52.12	1.069 0.900 0.788 1.41 55.46	0.604 0.557 0.520 0.01 45.93	0.967 0.841 0.771 0.88 52.69	0.788 0.745.0.745 0	0.919 0.793 0.649 0.47 53.23	1.039 0.609 0.536 3	1.006 1.033 0.689 0.97 55.03
FILE CST-5-20	0.5 0.6									
FILE	0.4	9.80	14.10	15.50	14.60	12,10	12.30	10.63	24.50	15.80
	SAMPLE # 02 03	RA203-192/219 93 0.010 0.010	8 8 2 9 3 - 2 4 3 / 2 7 0 0 . 6 1 4 1 4 . 1 0	8 4203-341/368 126 0,014 0.016 15.50	84207-005/032 78 0.014 0.015 14.60	. RA209-129/156 92 0.011 0.012 12.10	RA209-160/187 89 0.012 0.012 12.30	RE214-185/212 71 5-009 0-011 10-60	R8214-363/395 134 0.022 0.025 24.50	R8220-231/258 124 0.012 0.018 15.80